

Improved Performance of JPCP Through a Better Awareness of Plastic Shrinkage and Drying Shrinkage

Transportation Forum

Julie M. Vandebossche, P.E., Ph.D.
and Nicole Dufalla
University of Pittsburgh

February 5th, 2014

Shrinkage

- Plastic shrinkage cracking
 - Occurs in fresh concrete
 - Rate of evaporation exceeds surface water produced by bleeding
- Drying shrinkage
 - Effects slab shape
 - Influences fatigue stress



Approach

- Review
 - construction practices
 - specifications
- Lab study

[Finishing]



Wet burlap drag micro texture



Excess surface water added

Wet burlap drag micro texture



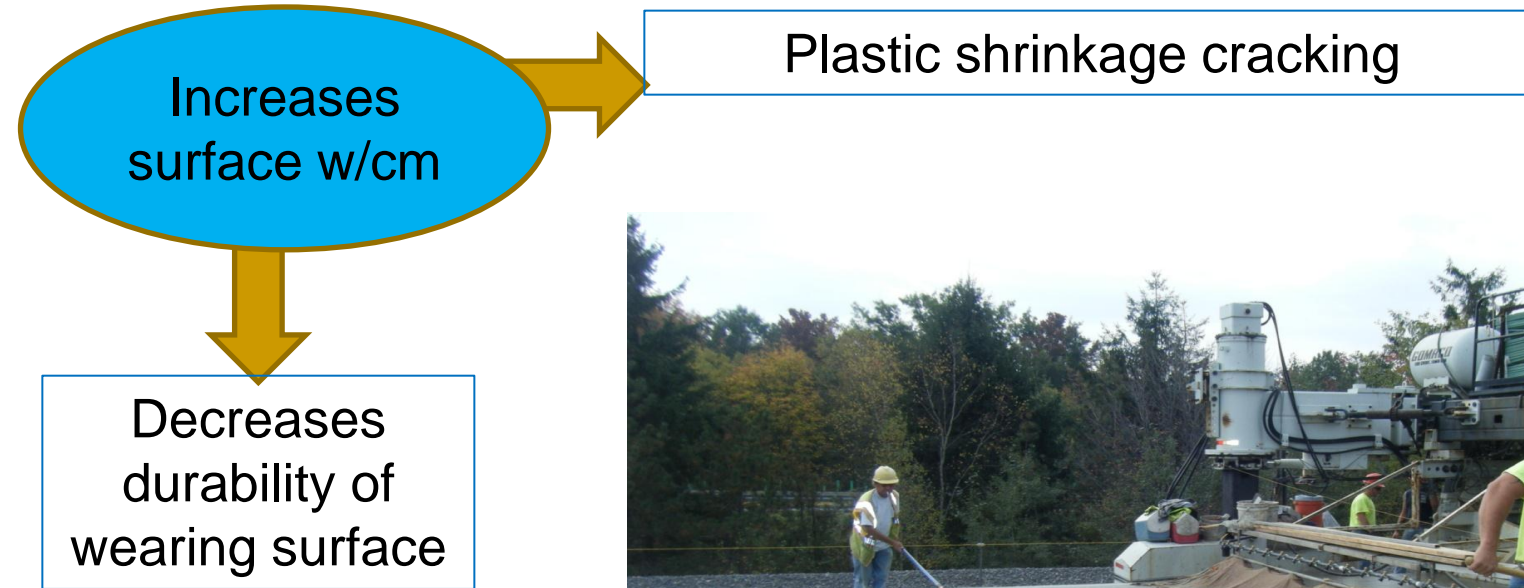
Excess surface water added

Finishing

- Excess surface water worked into surface
- Micro texture removed by finishers



Excess surface water



Plastic shrinkage cracking

- Effect on performance
 - Decreases surface durability
 - Potentially develop full depth if in central portion of the slab



Top View

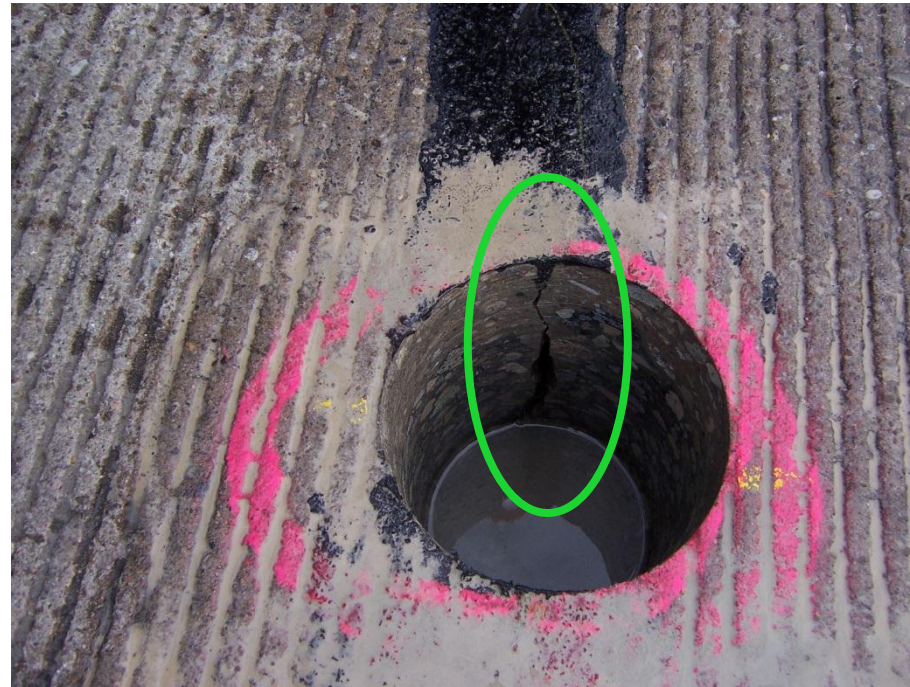


Side View

[Full depth cracking]



I-80 in Clinton County, PA



Plastic shrinkage cracking

I-80 in Clinton County, PA



(Crack has been digitally enhanced)



On core surface



(Cracks have been digitally enhanced)

WO12: 4 out of 6 projects exhibited plastic shrinkage cracking

[Polishing/erosion]

I-80



Vanport Limestone

SR-22



Recommendations

- Eliminate soaked burlap
- Texture after finishing
- Other micro texture options



Broom



Astroturf drag

Resulting texture



[Curing]



Curing compound application



Application with hand wand

Nonuniform



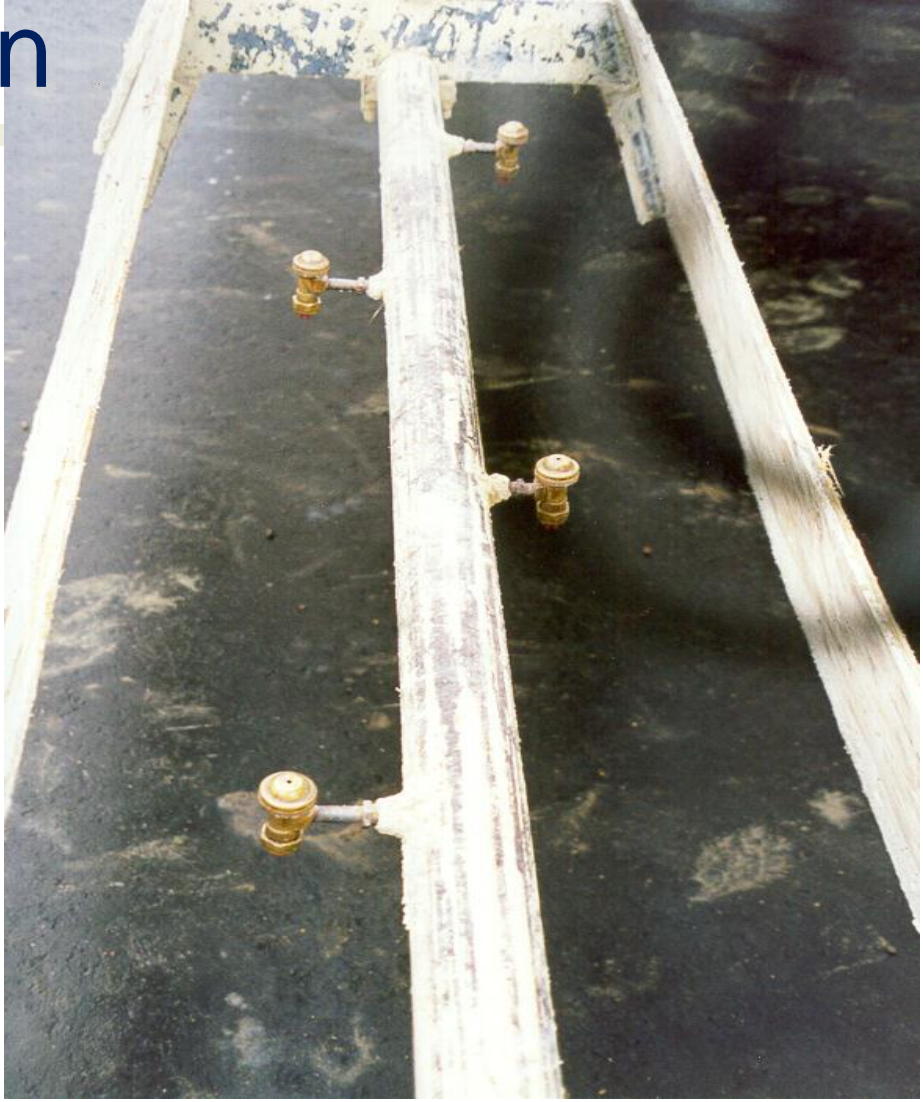
Curing compound application

PennDOT specification
requires curing cart



More uniform

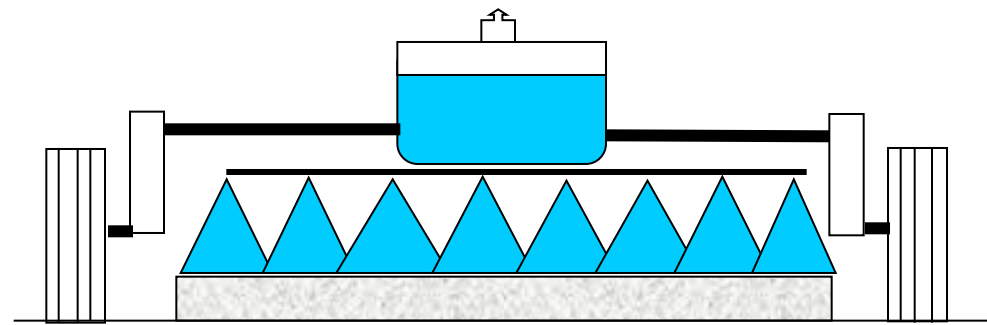
[Curing compound application



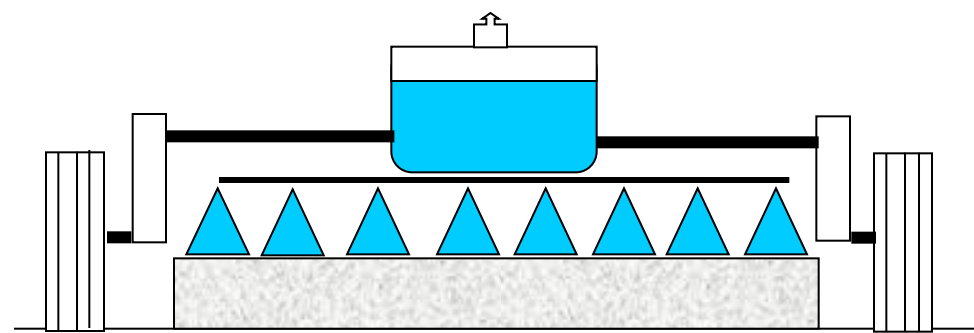
Curing compound application

- Nozzles must direct cure from 2 different lateral directions
- or.....
- 2 sets of nozzles along 2 traverse lines, each line capable of complete coverage

Coverage



8-in PCCP

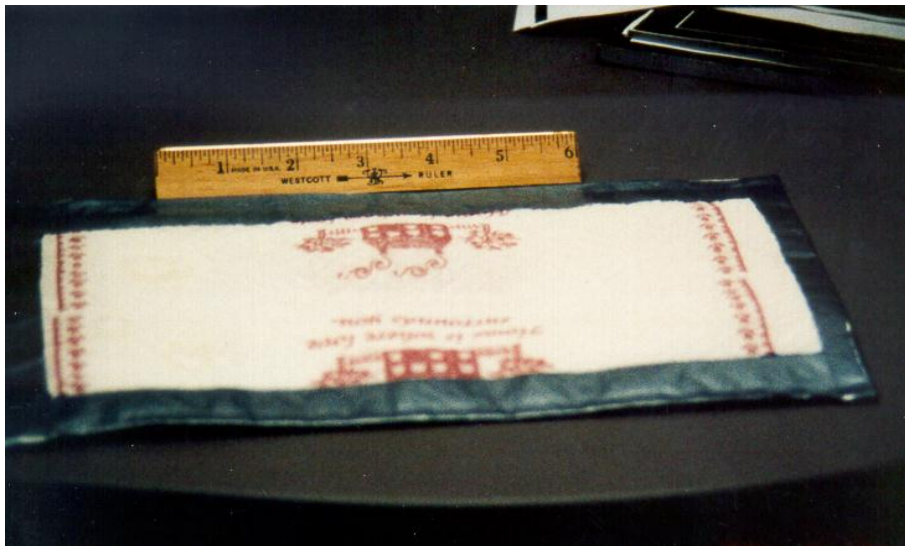


10-in PCCP



Application rate

Measured: **220 sq ft/ gallon**



PA spec.: **150 sq ft/ gallon**

Manufacturer's recommendations:
200 sq ft/ gallon

[Cart speed]

$$\text{Mph} = \frac{0.13636 \text{ GPM}}{\text{GPF} \times W}$$

Mph = miles per hour

GPM = gal. per minute per nozzle

GPF = gal. per ft²

W = nozzle spacing (in)

Example: 8004 nozzle
@ 40 psi pump
pressure



0.45 mph
(or 2400 ft/hr)



Curing compound storage

- Limit shelf life (1 yr from manufactured date)
 - Manufacturers recommendation
 - Don't allow to freeze



Effectiveness of curing compounds

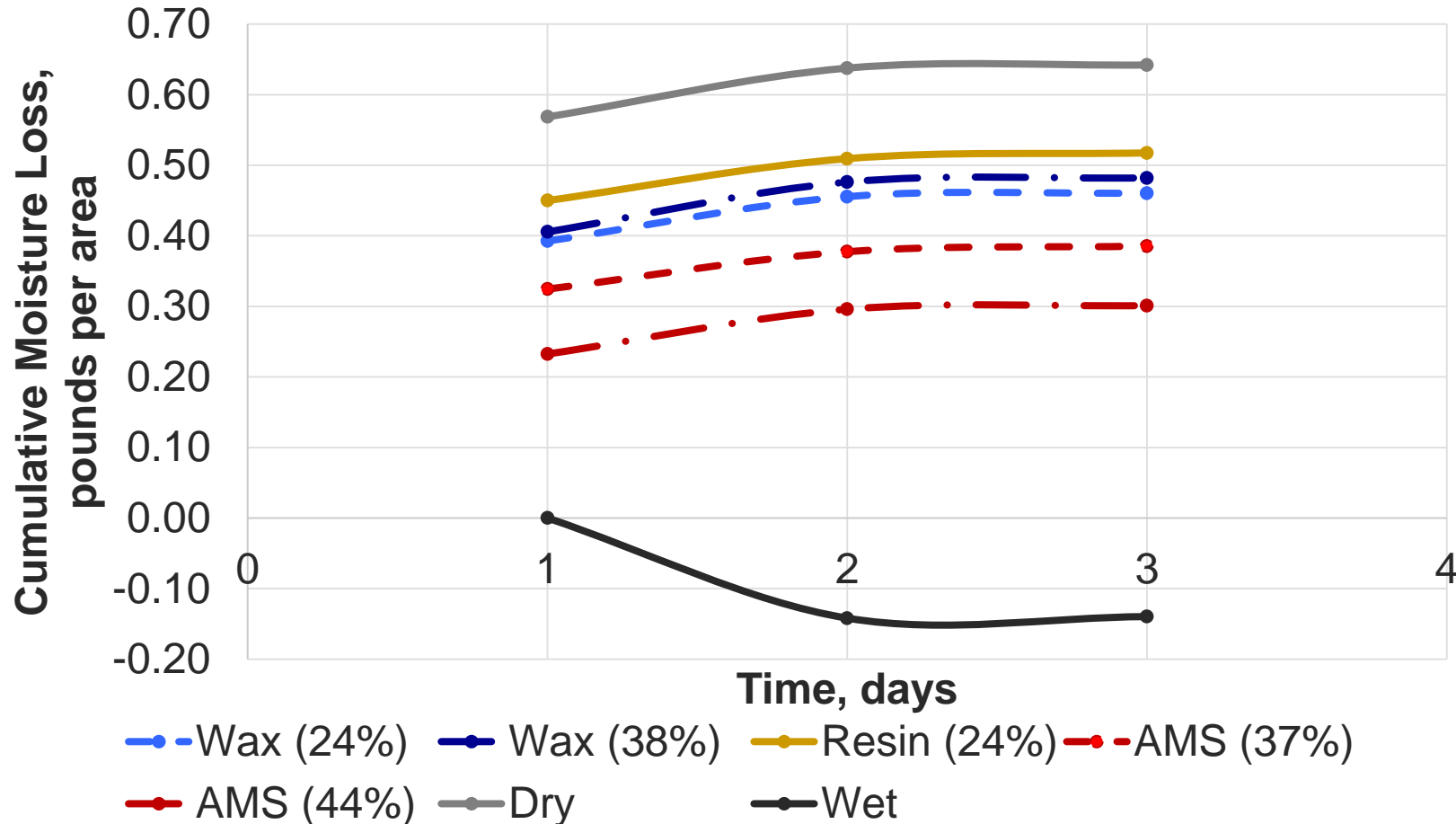
- Variables considered
 - Wax vs resin
 - Resin type (Poly alpha methylstyrene (AMS), other)
 - % solids
- Parameters measured
 - Moisture loss
 - Strength
 - Permeability



Moisture loss



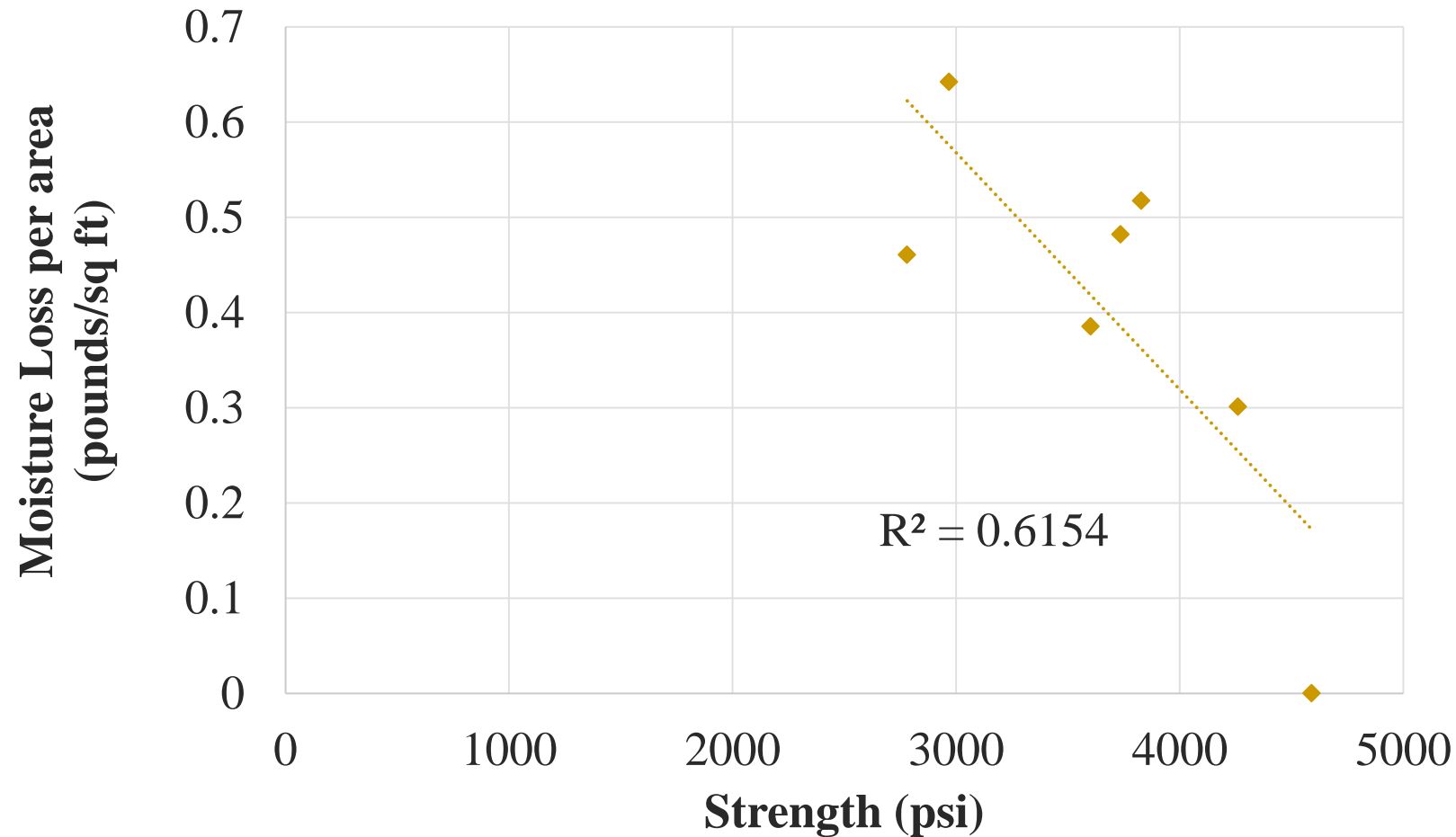
Moisture loss, 3-day



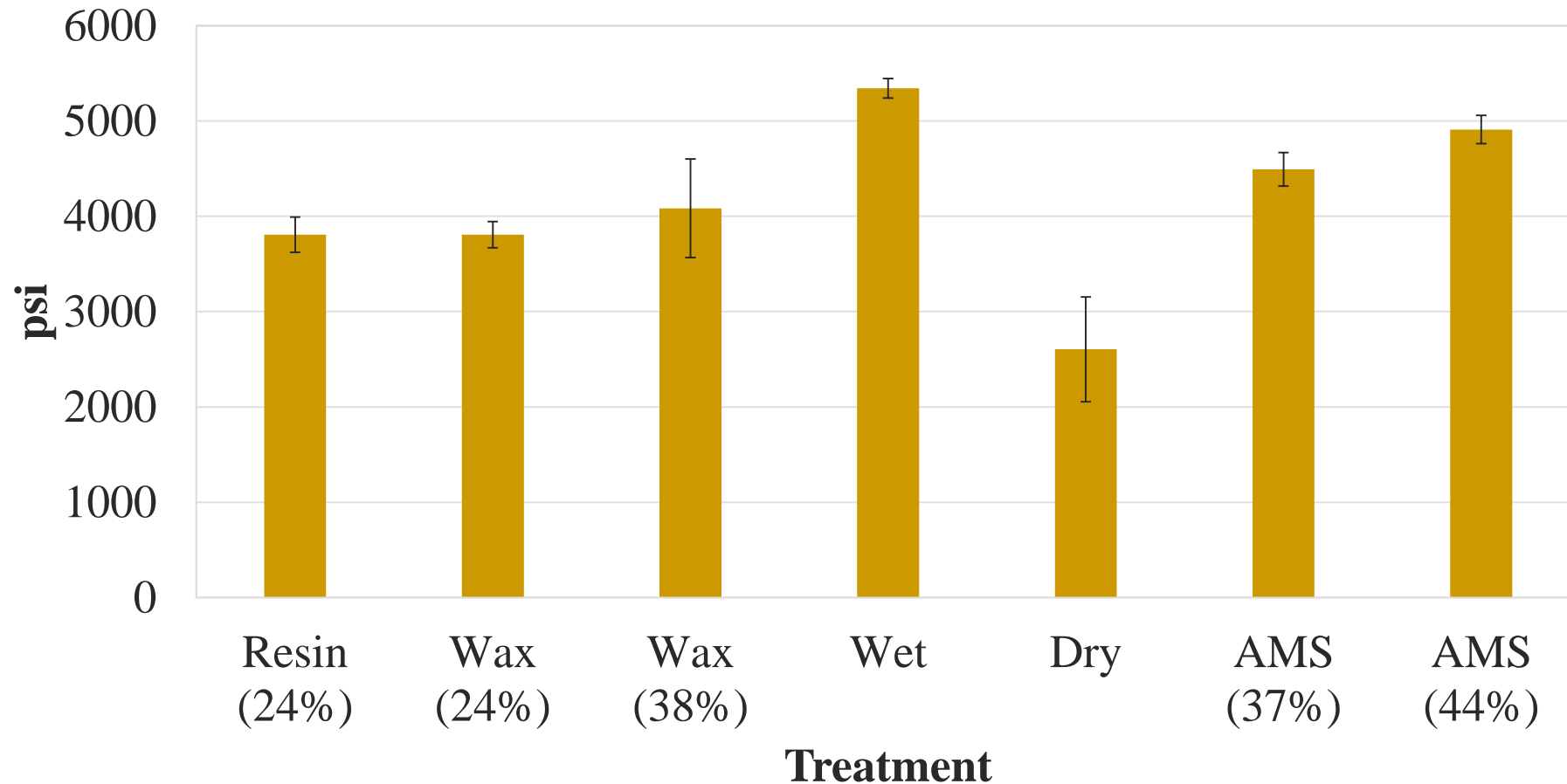
Performance



Compression strength, 3-day



Compression strength, 7-day



Recommendations- Curing

- 44% Poly alpha methylstyrene

Poly alpha methylstyrene requirements	
Total solids (% by weight)	42 min
% reflectance in 72 hours	65 min
Loss of water in 24 hrs, lb/sf	0.03 max
Loss of water in 72 hrs, lb/sf	0.08 max
VOC content, lb/gal	2.93 max
Infrared spectrum, vehicle	100% alpha methylstyrene



Mix design

Criteria	Pennsylvania
Max. w/c ratio	0.47
Max cement content	752 lb/cyd

4 PennDOT Projects evaluated in 2011:

w/cm = 0.45, 0.47, 0.49 and 0.46

Min. cement content (600 lbs) and w/c ratio (0.40) to increase durability and decrease shrinkage

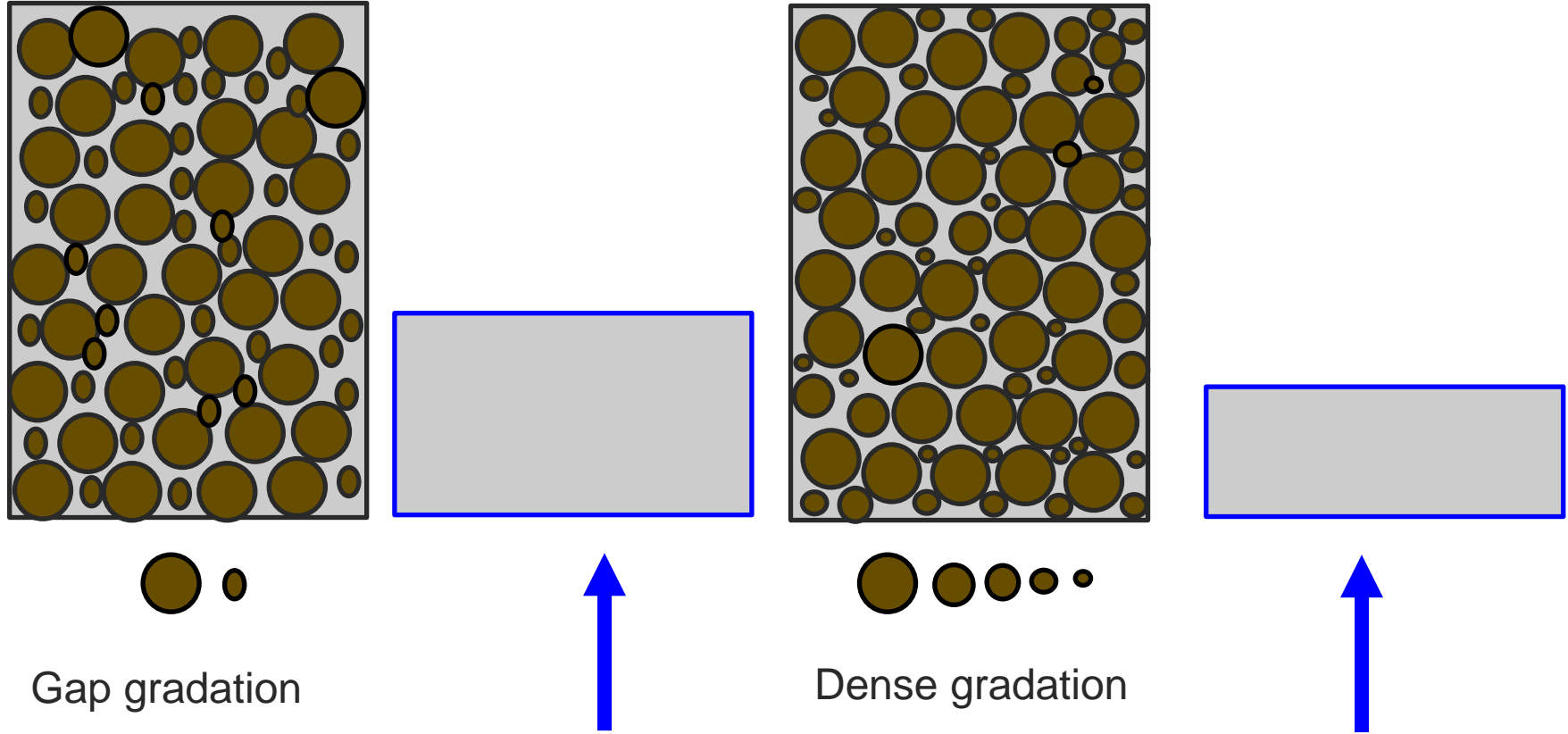
Coarse aggregate gradation

Sieve	MN	PA
1 ½ in	100	100
1 ¼ in	95-100	
1 in		95-100
¾ in	55-85	
½ in		35-60
3/8 in	20-45	
No. 4	0-7	0-10
No. 8		0-5

Specification allows for gap gradation

- Higher paste demand
- Segregation

Cement demand



Gap gradation

Dense gradation

Volume of paste required to fill voids



Segregation



SR 202
Chester county, PA



SR 22
Murrysville, PA



I-79
Washington county, PA

Observed frequently for WO12

Coarseness factor chart

Zone I: Coarse gap-graded
tends to segregate

Zone II: Well graded 2"
thru 3/4"

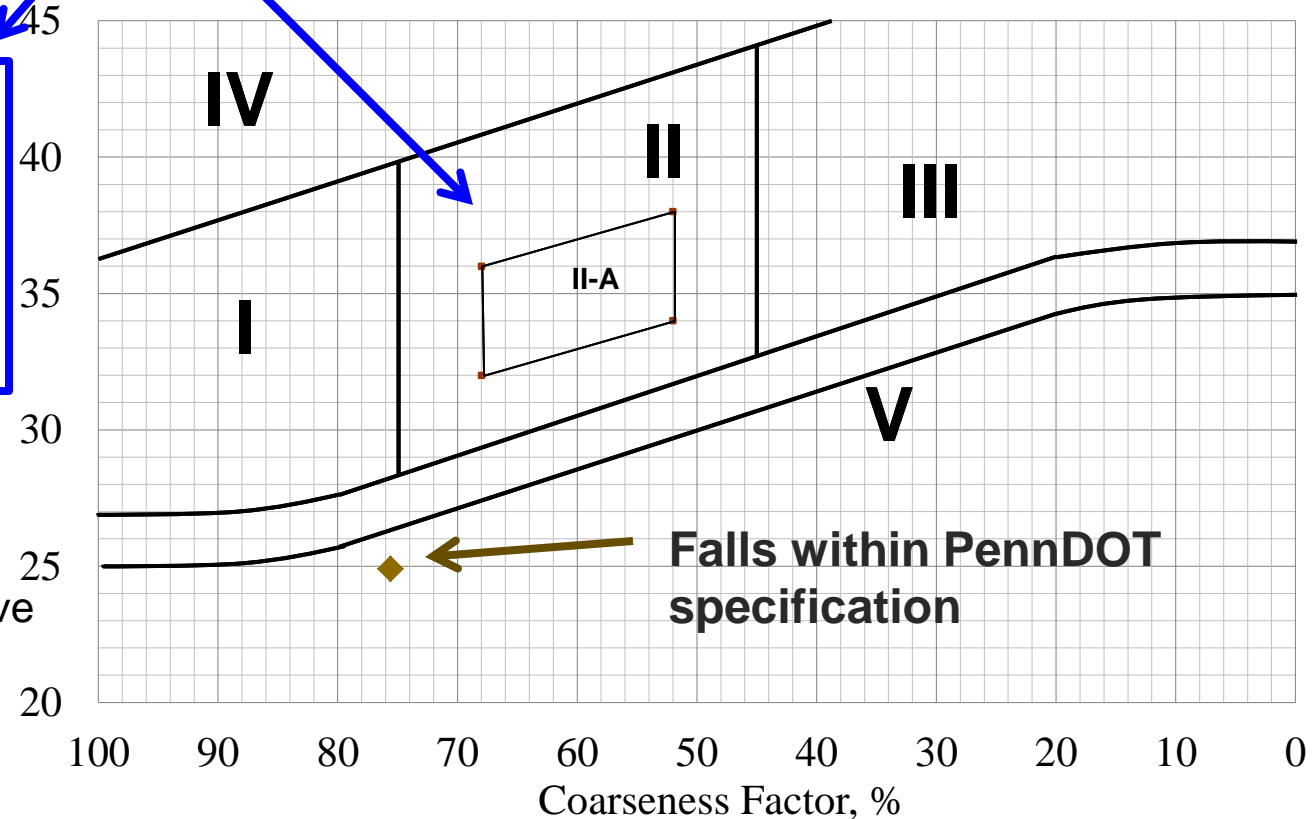
Zone II-A: Well-graded
aggregate gradation
incentive

Zone III: 3/4" minus
mixtures

Zone IV: Sandy - excessive
fines

Zone V: Rocky - non-
plastic

Ideal gradation zones

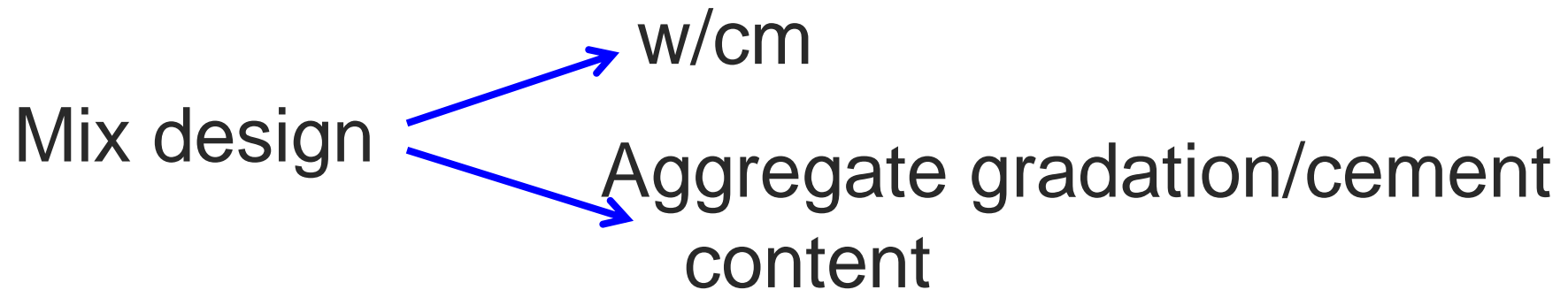
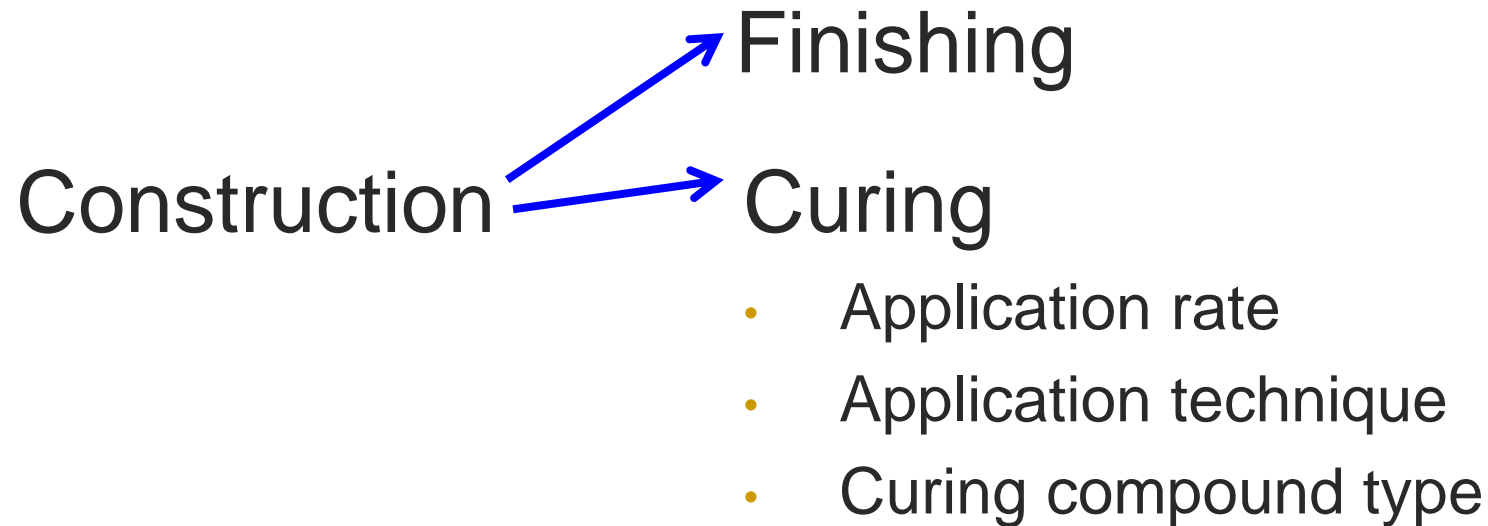


Recommendations- Mix design

- Minimize cement content
 - Adopt more densely graded aggregate
- w/cm ratio
 - New target of 0.40
 - Should fall between 0.38 and 0.42



Critical areas



Acknowledgements

Project coordination and technical assistance

- Mr. Steven Marsinko, Mr. Leonard Kubitza and Mr. William Kovach

Site visits

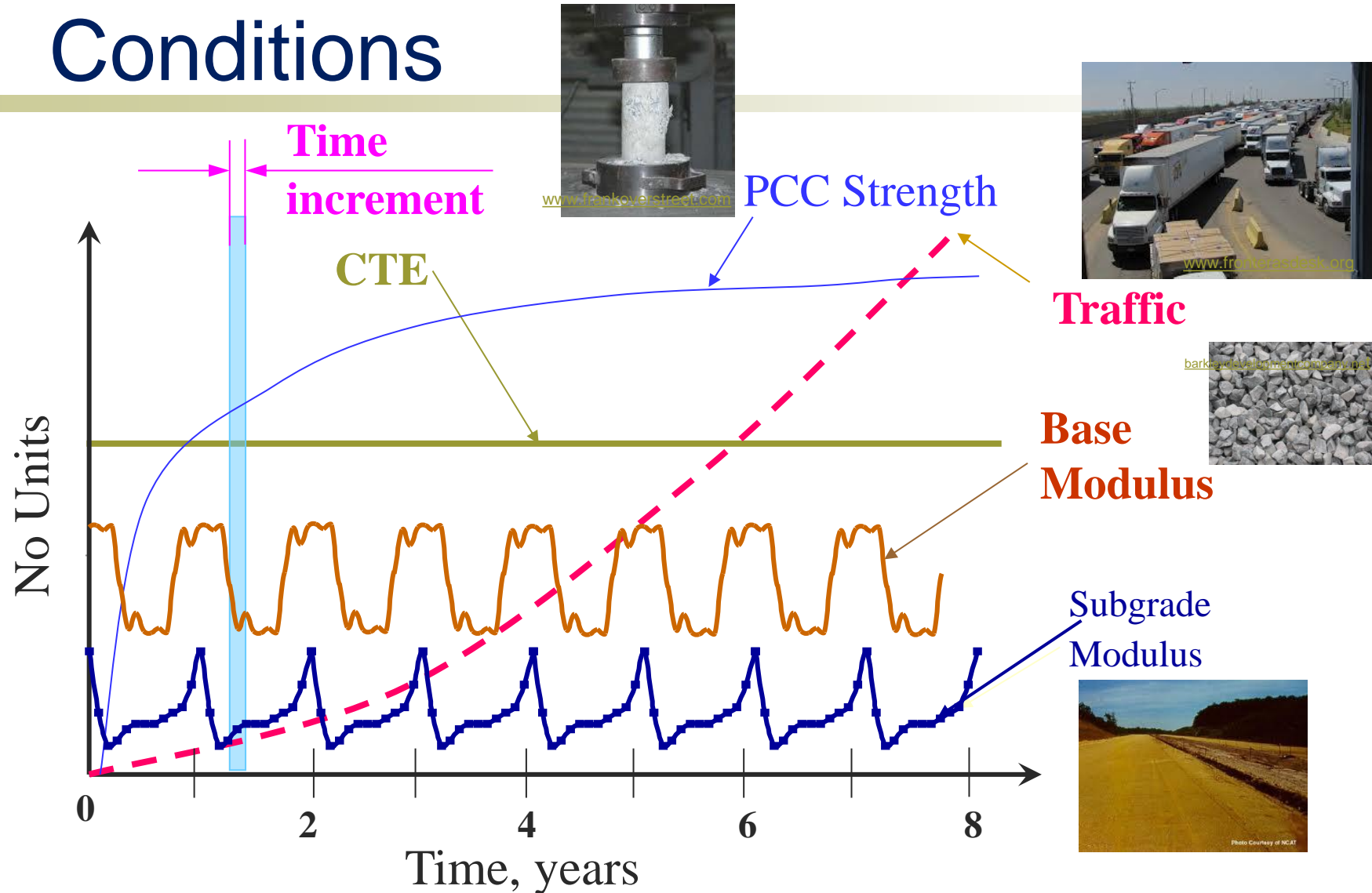
- Mr. William Snyder, Ms. Roxanne Rossi, and Mr. David Schaeffer



National Calibration of PCC Performance
Models in AASHTO ME Pavement
Design
NCHRP Project 20-07/Task 327

Julie M. Vandebossche, P.E., Ph.D.
Steven Sachs
University of Pittsburgh

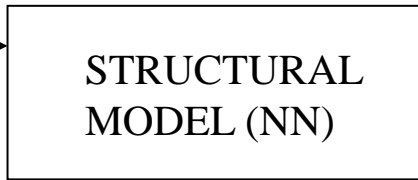
Models Consider Changing Conditions



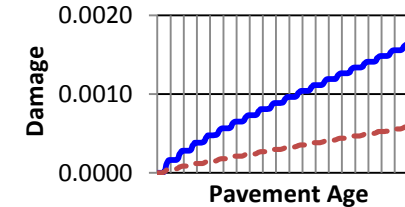
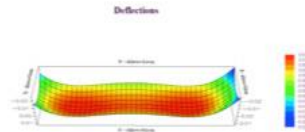
New AASHTO Design PAVEMENT ME

INPUTS

Slab thickness
k-value
Axle Loads and Volumes
PCC M_r
Climate



σ, ϵ, δ

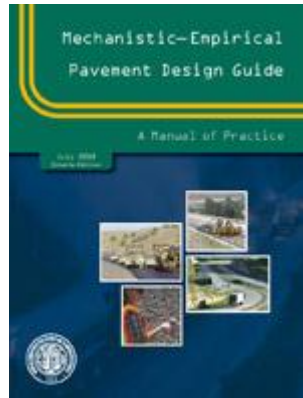


$$\log(N) = C1 \cdot \left(\frac{MR}{\sigma}\right)^{C2}$$

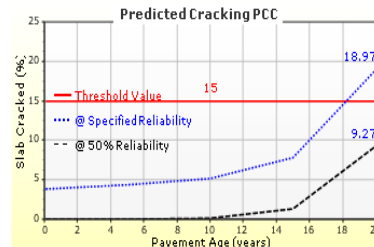
$$CRK = \frac{100}{1 + C4 FD^{C5}}$$

CALIBRATION WITH FIELD DISTRESS

OUTPUTS
Performance Prediction:
Cracking, Faulting, IRI



www.darwinme.org



www.fhwa.dot.gov



JPCP joint faulting

$$FMAX_0 = (C_1 + C_2 * FR^{0.25}) * \delta_{curl} * \left[\text{Log}(1 + C_5 * 5^{EROD}) * \text{Log}\left(\frac{P_{200} * WetDays}{p_s}\right) \right]^{C_6}$$

$$FMAX_i = FMAX_{i-1} + C_7 * DE_i * \left[\text{Log}(1 + C_5 * 5^{EROD}) \right]^{C_6}$$

$$\Delta Fault_i = (C_3 + C_4 * FR^{0.25}) * (FMAX_{i-1} - Fault_{i-1})^2 * DE_i$$

$$Fault_i = Fault_{i-1} + \Delta Fault_i$$

$$\Delta DOWDAM_{tot} = \sum_{j=1}^3 \sum_{i=1}^N C_8 * F_{ij} \frac{n_{ij}}{df_c^*}$$



M-E PDG (2004)
(NCHRP 1-37A)

Original Performance
Model Calibrations

M-E PDG (2006)
(NCHRP 1-40)

Recalibrated
Performance Models

- Expanded datasets
- Updated software

Pavement ME (2013)

Functionally same version
of the software

DARWin-ME (2011)

AASHTOWare software
version

- Improved GUI
- Decreased run times



Error in estimating Coefficient of Thermal Expansion (CTE)

- 304 SS CTE inaccurate over typical PCC service temp.s
 - $0^{\circ}\text{C} - 500^{\circ}\text{C} \rightarrow 17.3 \cdot 10^{-6}/^{\circ}\text{C}$
 - $10^{\circ}\text{C} - 50^{\circ}\text{C} \rightarrow 15.8-16.2 \cdot 10^{-6}/^{\circ}\text{C}$
- Steel CTE too high \rightarrow PCC CTE too high

LTPP database populated w/ incorrect CTEs
Models need recalibration with correct CTEs



[Importance of CTE for pavements]

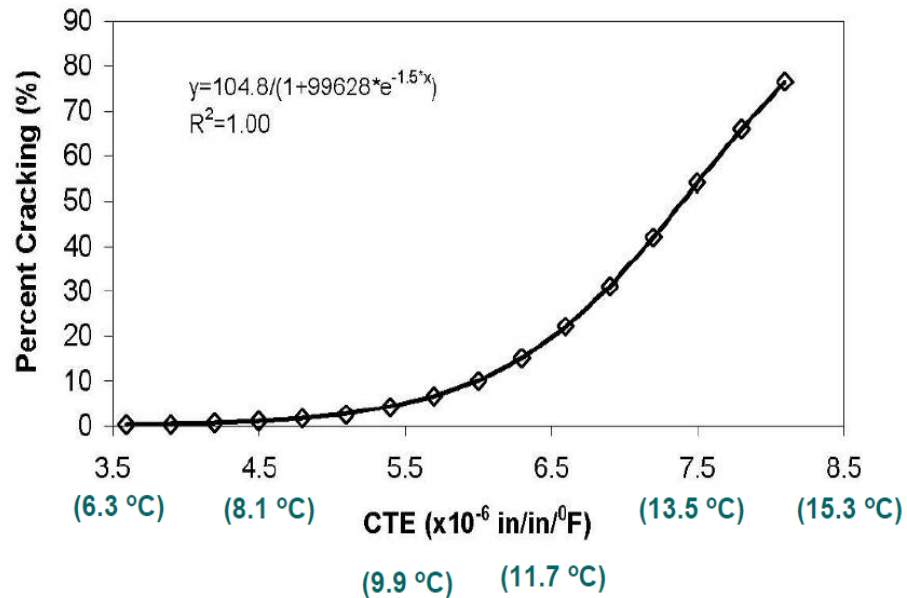
Can contribute to:

- LTE, Spalling & Faulting → Joint opening
- % Cracking Cracking → Thermal curling

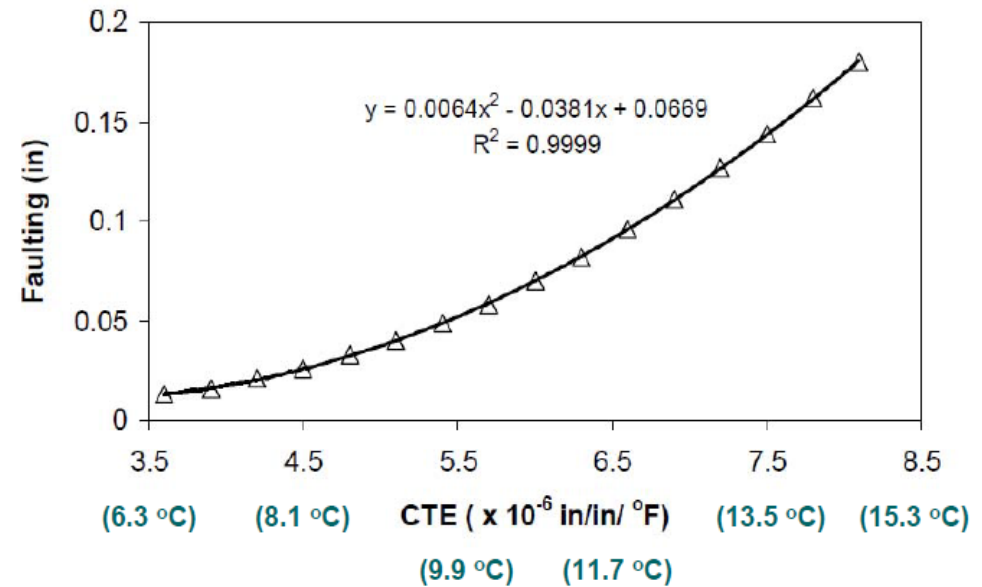


Pavement ME Design Guide

Sensitivity to CTE



Effect of CTE variability of Concrete Pavement Performance as predicted using the Mechanistic - Empirical Pavement Design Guide, Jussara Tanesi, M. Emin Kutay, Ala Abbas, and Richard Meininger, Transportation Research Board 2007.



Effect of CTE variability of Concrete Pavement Performance as predicted using the Mechanistic - Empirical Pavement Design Guide, Jussara Tanesi, M. Emin Kutay, Ala Abbas, and Richard Meininger, Transportation Research Board 2007.



[Recalibration]

NCHRP Project 20-07/Task 288 initiated

- Recalibrated:
 - JPCP (transverse cracking and faulting)
 - CRCP (crack width and punchout)
- Predicted slab thickness should be similar to original models
- Not implemented due to discrepancies in thickness designs

Previous National Calibration	1-37A	Task 288
*Average Predicted Slab Thickness (in)	11.37	10.57

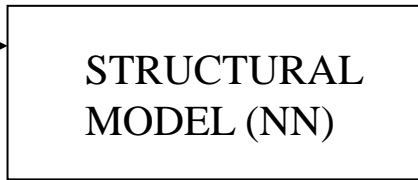
*Avg of sites w/in cracking factorial design (slab thickness required to meet default performance limits)



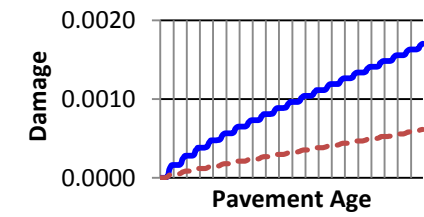
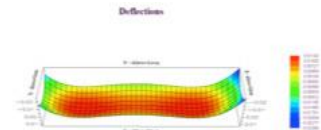
New AASHTO Design PAVEMENT ME

INPUTS

Slab thickness
k-value
Axle Loads and Volumes
PCC M_r
Climate



σ, ϵ, δ

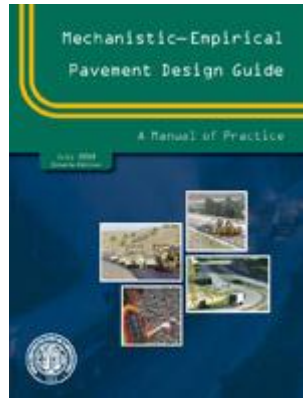


$$\log(N) = C1 \cdot \left(\frac{MR}{\sigma}\right)^{C2}$$

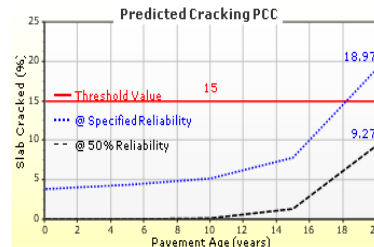
$$CRK = \frac{100}{1 + C4 FD^{C5}}$$

CALIBRATION WITH FIELD DISTRESS

OUTPUTS
Performance Prediction:
Cracking, Faulting, IRI



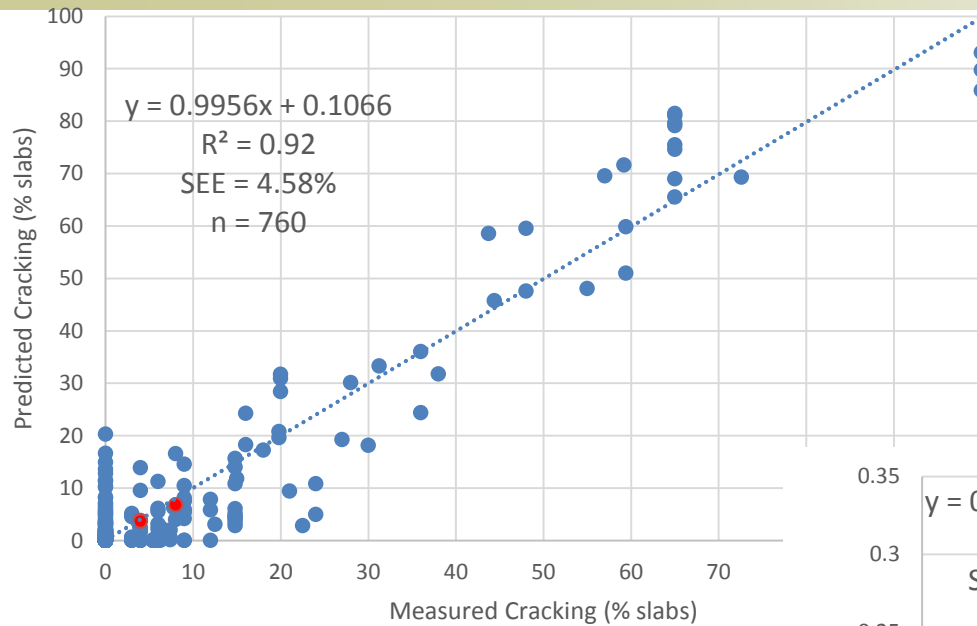
www.darwinme.org



www.fhwa.dot.gov

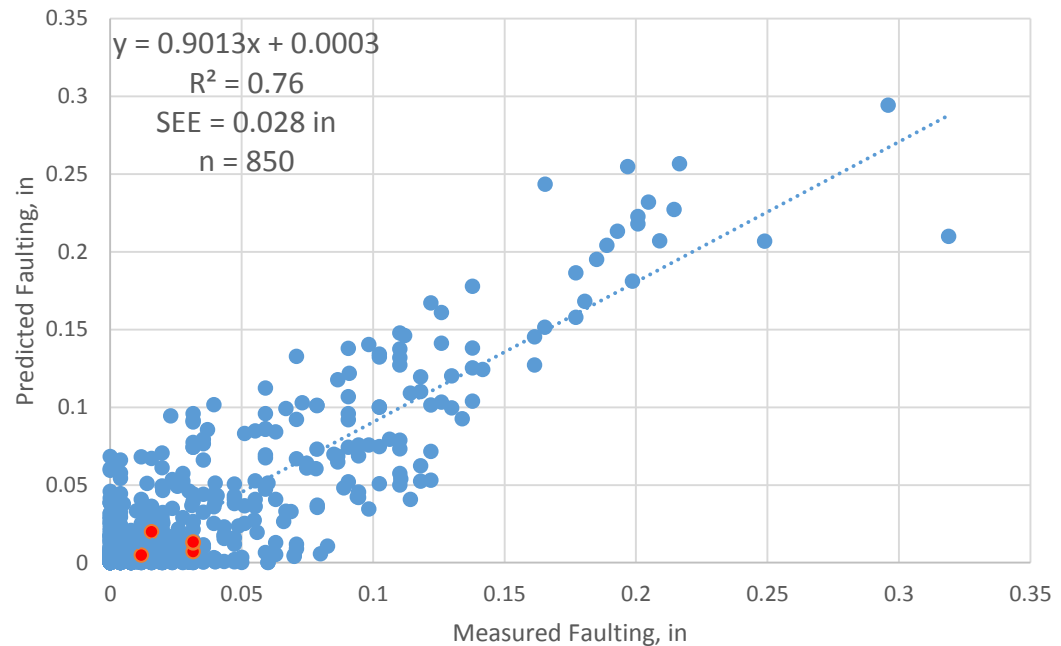


PA LTPP included in national calibration



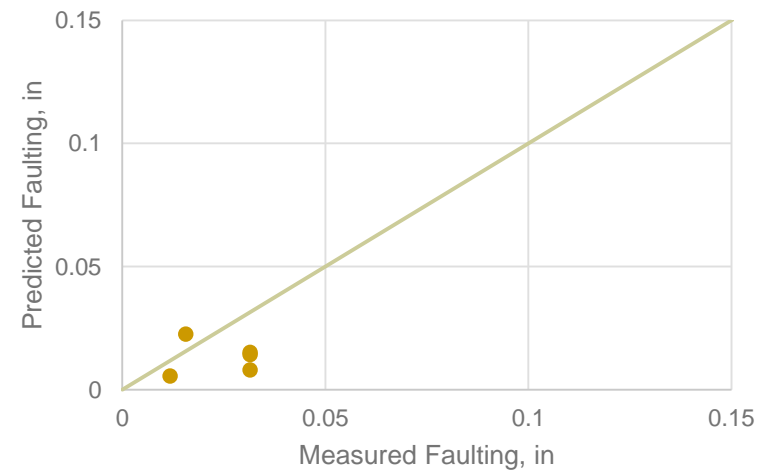
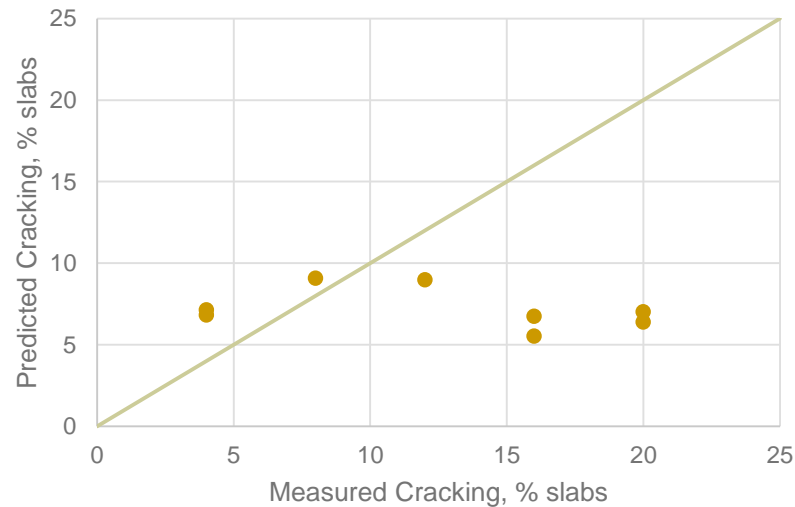
National calibration performed as part of NCHRP 20-07 Task 327.7

PA site highlighted in red

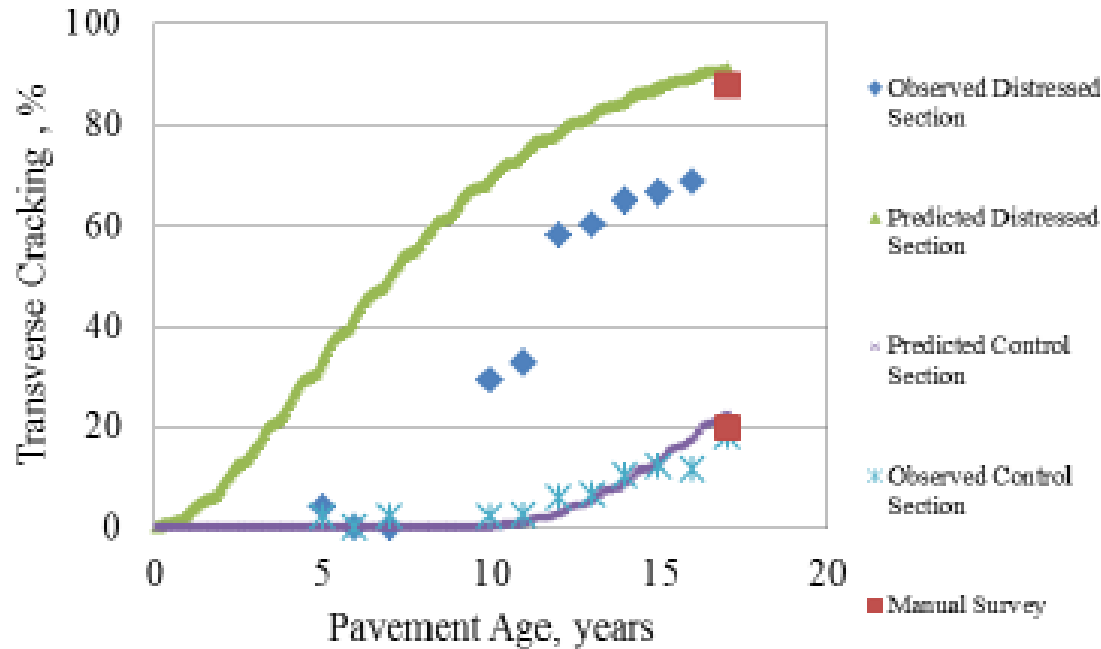


PA LTPP Sites

- Only 2 JPCP sites in national calibration database
- Principal Arterial – Interstate
- Part of GPS-3 (JPCP)

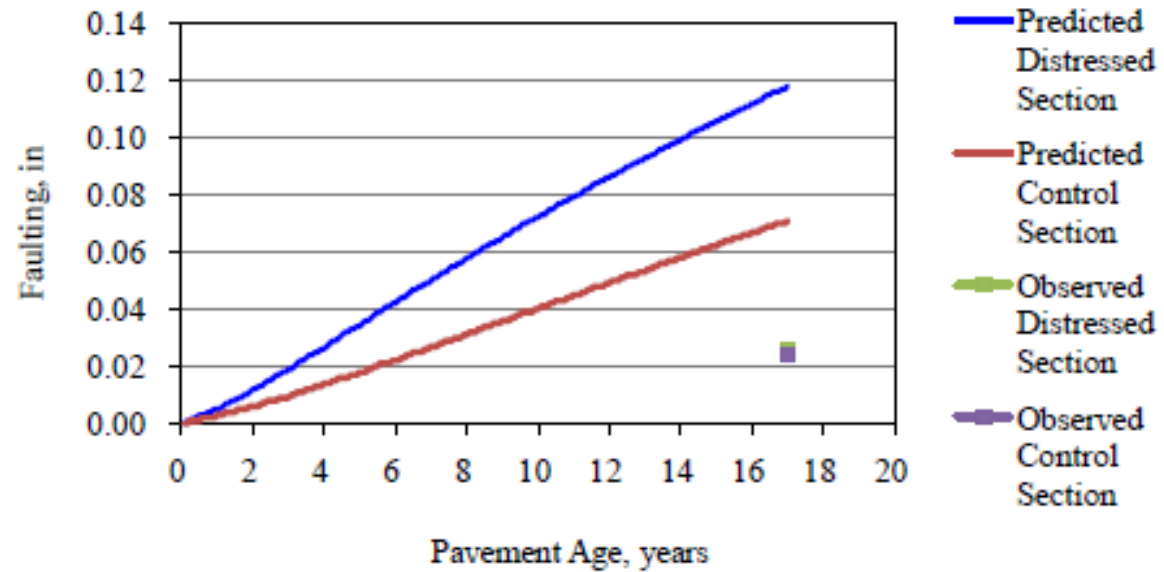


PA Predicted vs. Measured



**SR60:
Segments 323 and 303**

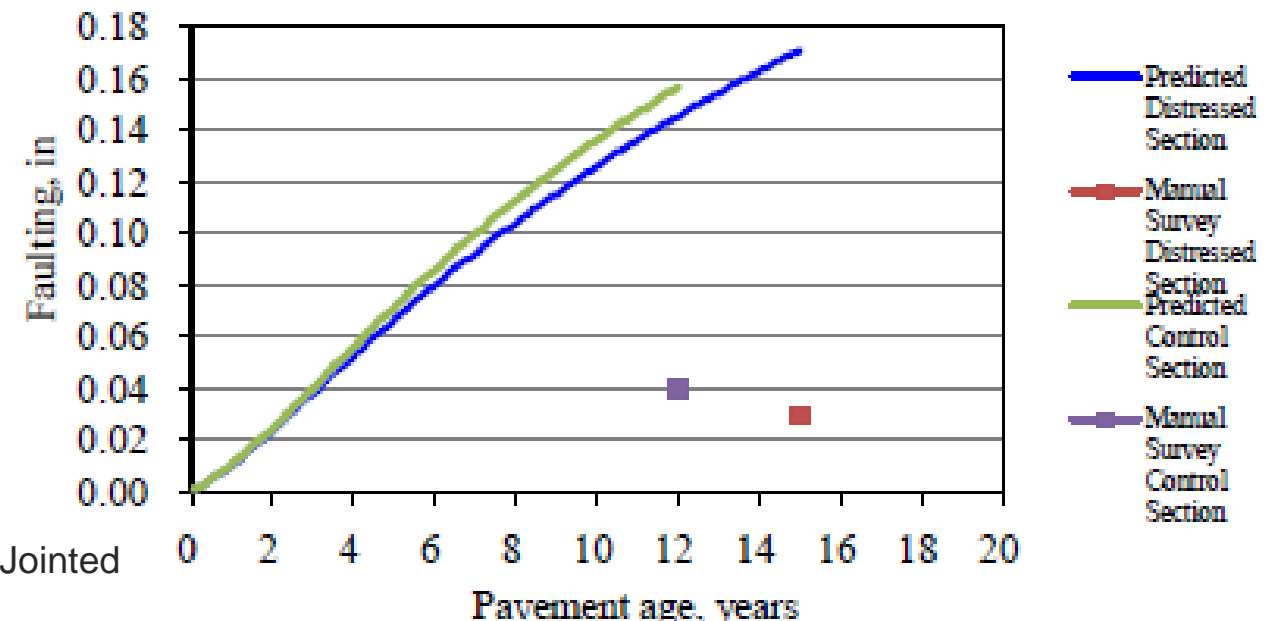
From PennDOT WO # 12
Report:
Premature Deterioration of Jointed
Plain Concrete Pavements



PA Predicted vs. Measured



I-79: Segments 470 and 480

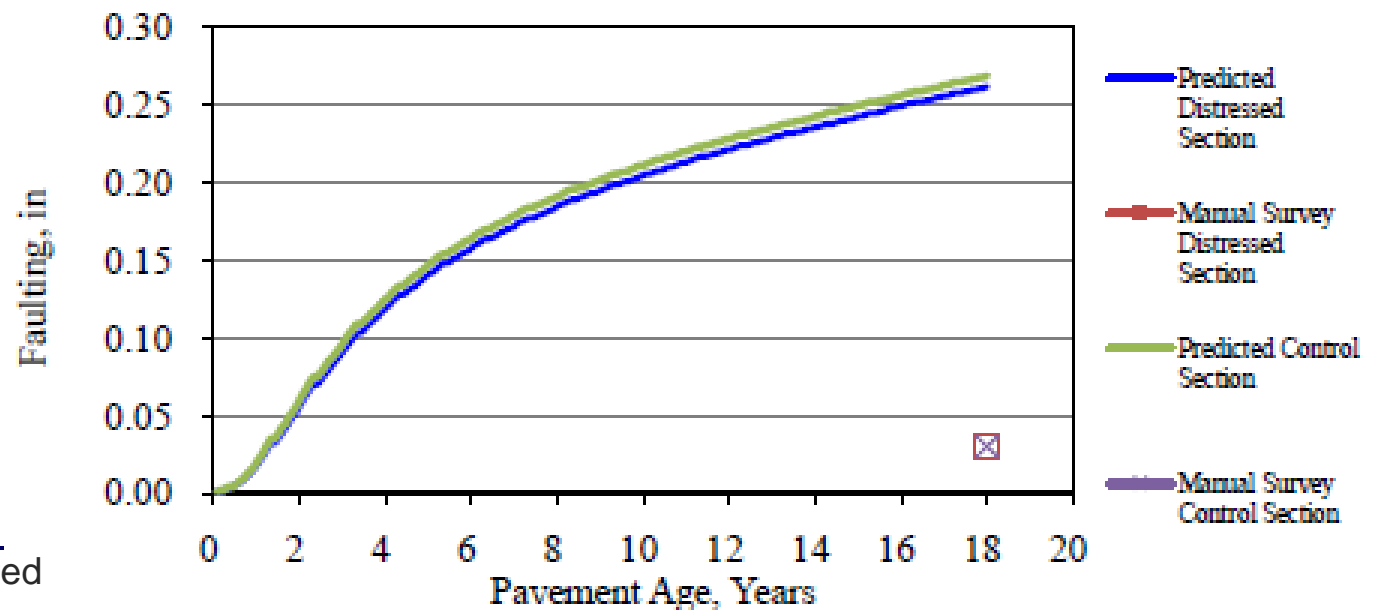
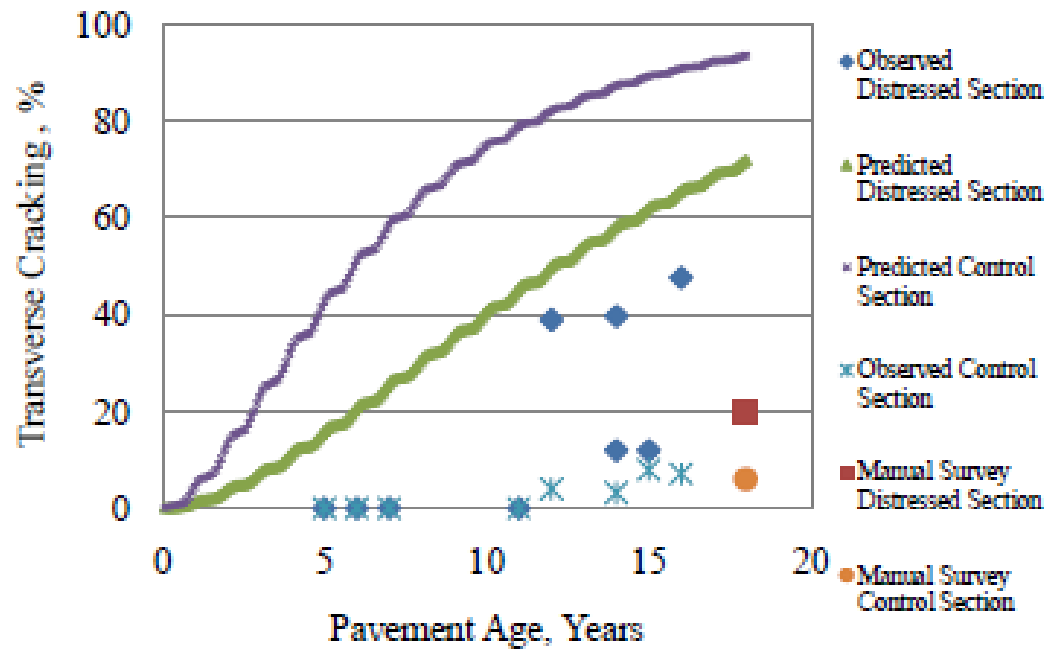


From PennDOT WO # 12 Report:

Premature Deterioration of Jointed Plain Concrete Pavements



PA Predicted vs. Measured



From PennDOT WO # 12
Report:
Premature Deterioration of Jointed
Plain Concrete Pavements

PA Local Calibration

- Representative date for recalibration
- Friction values
- Built-in gradient



Available PA calibration data

- 29 JPCP sections in PA
- Only 15 have distress data available
- Of those 15 sites, only 5 were not repaired with patching or another rehabilitation



Thank You



Any Questions?

Email: jmv7@pitt.edu