



University of Pittsburgh



pennsylvania
DEPARTMENT OF TRANSPORTATION

Bridge Waterproofing Details

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Overview of Presentation

- Introduction
- Bridge Waterproofing – Design and Details
- Review of Designs from different Departments of Transportation
- Field Visits
- Research Conducted
- Conclusions



Part 1

INTRODUCTION



About the Issue

- Bridges very important – Lifeline of the economy
- Maintenance of bridges expensive – both in cost of maintenance and productivity lost due to closure or diversion of traffic
- Leakage in bridges in Pennsylvania – prevalent early in the life of a bridge (within 5 yrs.)

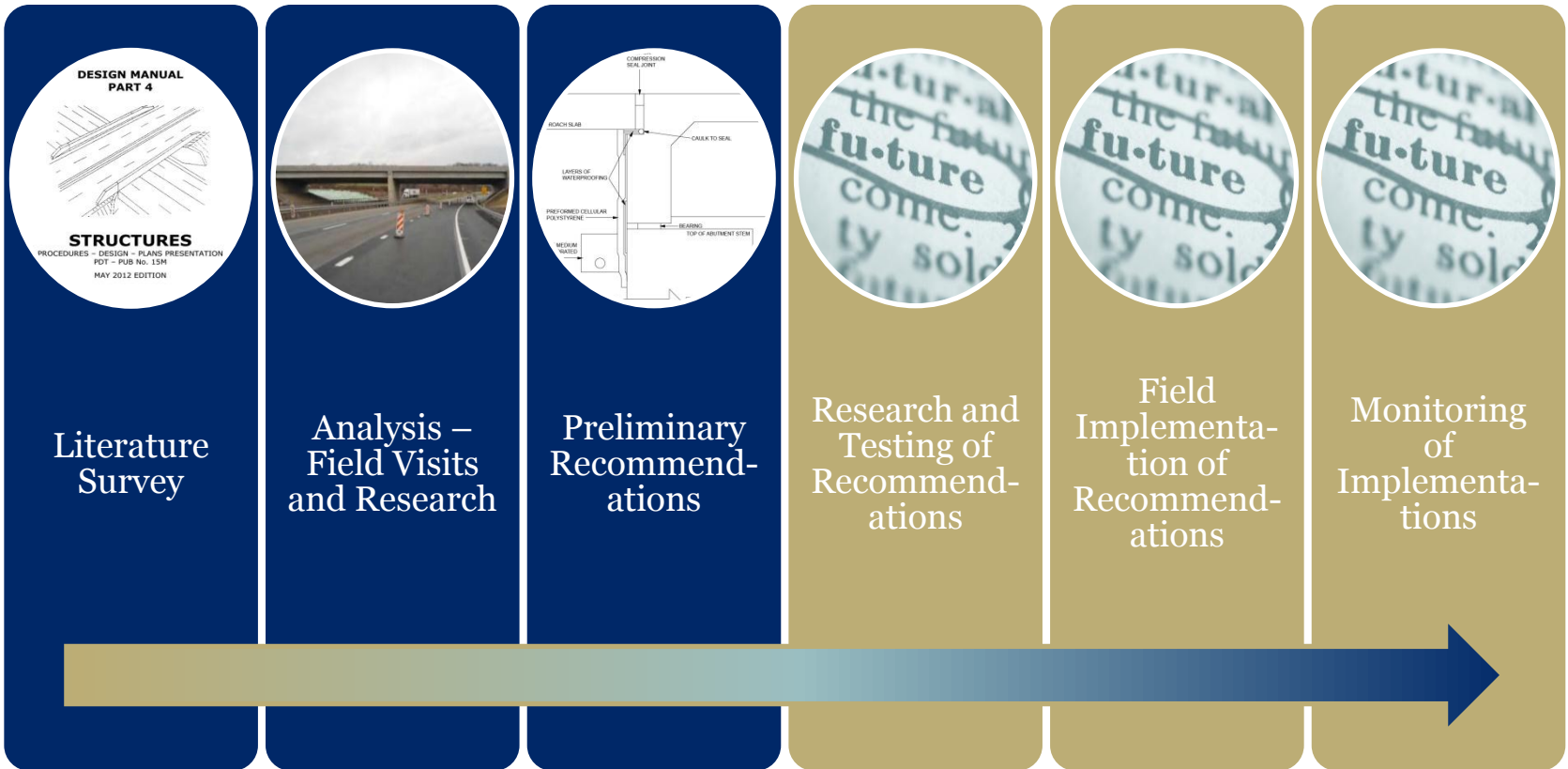


About the Issue

- Deteriorates life of the entire bridge due to issues such as efflorescence widening cracks, corrosion, stagnating water at bridge seats, etc.
- Usually caused due to failure of one or two small components
- Aim: To extend life of components of the bridge tending towards the life of the entire bridge



Where we stand...





Part 2

BRIDGE WATERPROOFING – DESIGN AND DETAILS



Research Outline

Entire research is divided into 3 parts

- Literature Review
 - Current abutment waterproofing implementation in PennDOT, ODOT, MassDOT & MnDOT
 - Current expansion joint implementation in PennDOT, ODOT, MassDOT, MnDOT, IDOT & NYSDOT
 - Current inspection procedures in PennDOT, ODOT, MassDOT & MnDOT



Research Outline

- Field Visits and Research based Analysis
 - Analysis of bridges in PennDOT, MassDOT & MnDOT based on inspection reports
 - Field analysis of 5 operational bridges and 1 under construction bridge in Pennsylvania
 - Simulated and Experimental analysis of critical parts of waterproofing system
- Recommendations
 - Based on inferences from field visits and research based analysis
 - Based on experience and information of engineers from multiple DOTs



Bridge Waterproofing

- Components to protect bridge structure from coming in prolonged contact with water:
 - Expansion joints
 - Abutment waterproofing (in the backfill area) and drainage
 - Deck waterproofing and drainage
- In this research, we do not focus on waterproofing of the deck and focus on the other areas because it is beyond the scope of the current project



Expansion Joints

- A non-structural component to accommodate the movement of the deck due to:
 - Concrete shrinkage and creep
 - Post-tensioning shortening
 - Thermal variations
 - Dead and live loads
 - Wind and seismic loads
 - Structure settlements
- Also provides ride comfort, *prevents runoff water* and deicing chemicals from *leaking onto bearings, abutments*, and other structural elements underneath the bridge deck



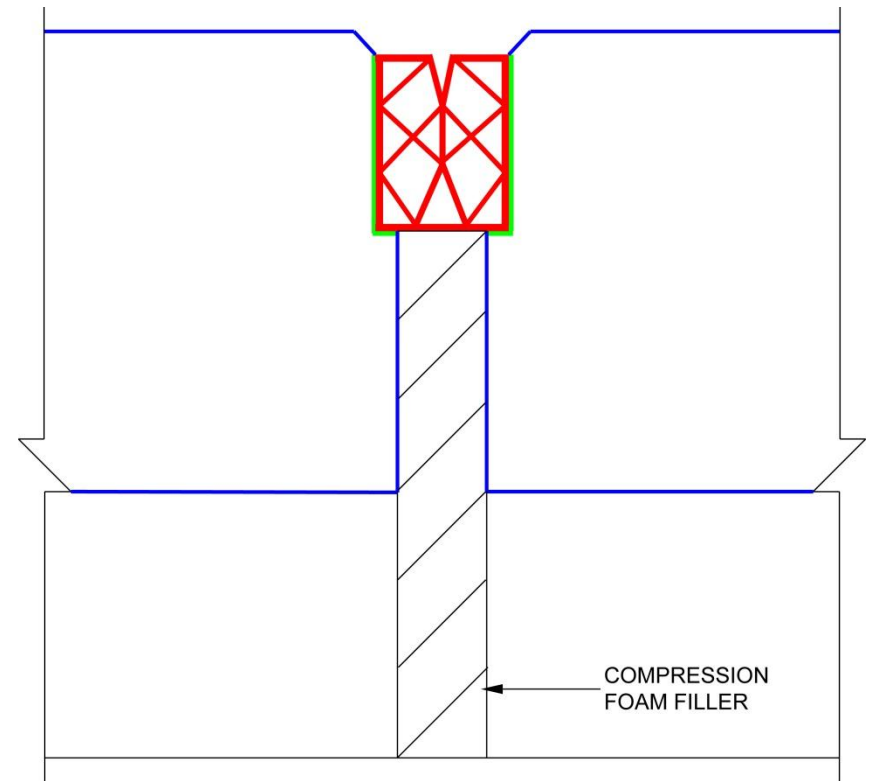
Expansion Joints

- 5 main types of expansion joint systems used:
 - Compression Seal Joint
 - Strip Seal Joint
 - Tooth Expansion Dam
 - Modular Bridge Expansion Joint (MBEJ)
 - Asphaltic Plug Joint
- Research focuses on Compression and Strip seal joints
 - Most common types of joint systems used
 - Allow for relatively small expansion and thus have smaller tolerances for difference between designed (predicted) joint opening and movement, and actual joint opening and movement
 - Tooth Expansion Dam and MBEJ are designed to carry traffic loading and are thus less susceptible
 - Asphaltic Plug Joint is mainly used in temporary fixes



Compression Seal Joint

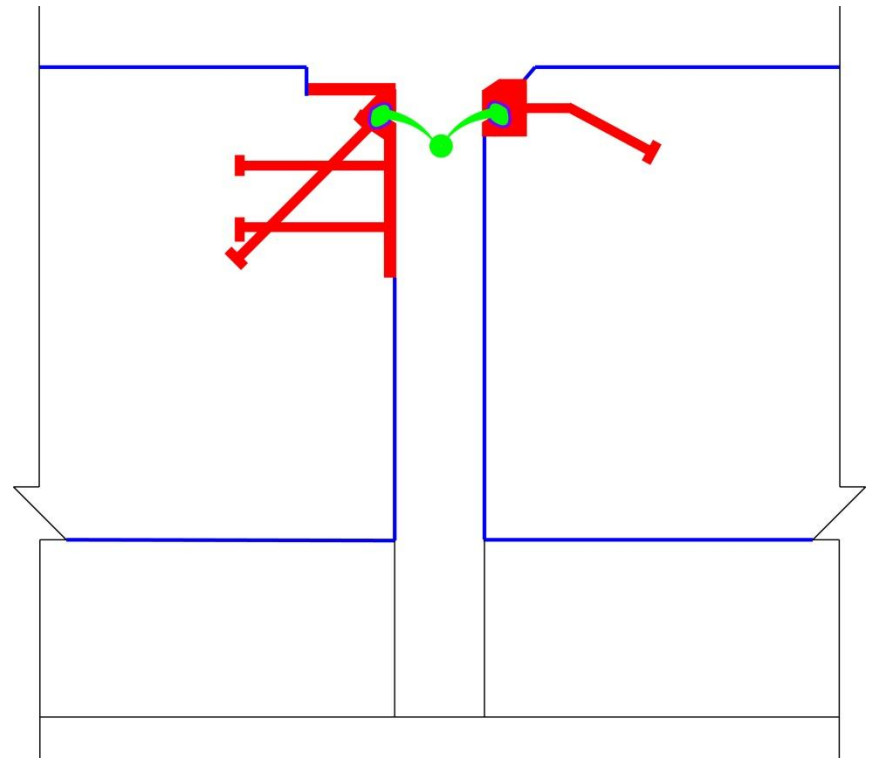
- Movement Range: 0.5" – 3.0"
- Components:
 - Compression Seal
 - Lubricant-adhesive
 - Block-out or Saw-cut opening
- Note: CSJ steel extrusion and anchorage are not considered – not used in PennDOT





Strip Seal Joint

- Movement Range:
0.5" – 3.0"
- Components:
 - Strip Seal
 - Steel Extrusion and Anchorage
 - Lubricant Adhesive
 - Block-out





Comparison of Compression and Strip Seal Joints

- Compression seal joints are relatively cheap to install and repair than strip seal joints; Also requires lesser time to install and can be done in parts
- Strip seal joints have a better lifespan than compression seal joints
- Strip seal allows for greater skew angles and is more tolerant to difference between predicted and actual joint opening and movement; Also very tolerant to occasional traffic loading



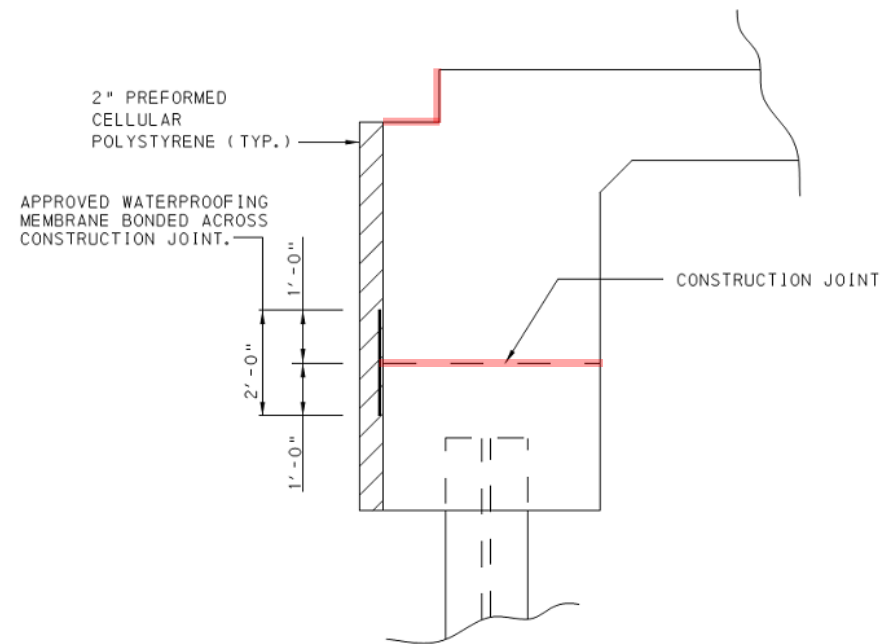
Abutments

- Supports superstructure – retaining wall holds backfill
- Two types: Connection between abutment stem and bridge superstructure
 - Integral/Semi-Integral
 - Parapet



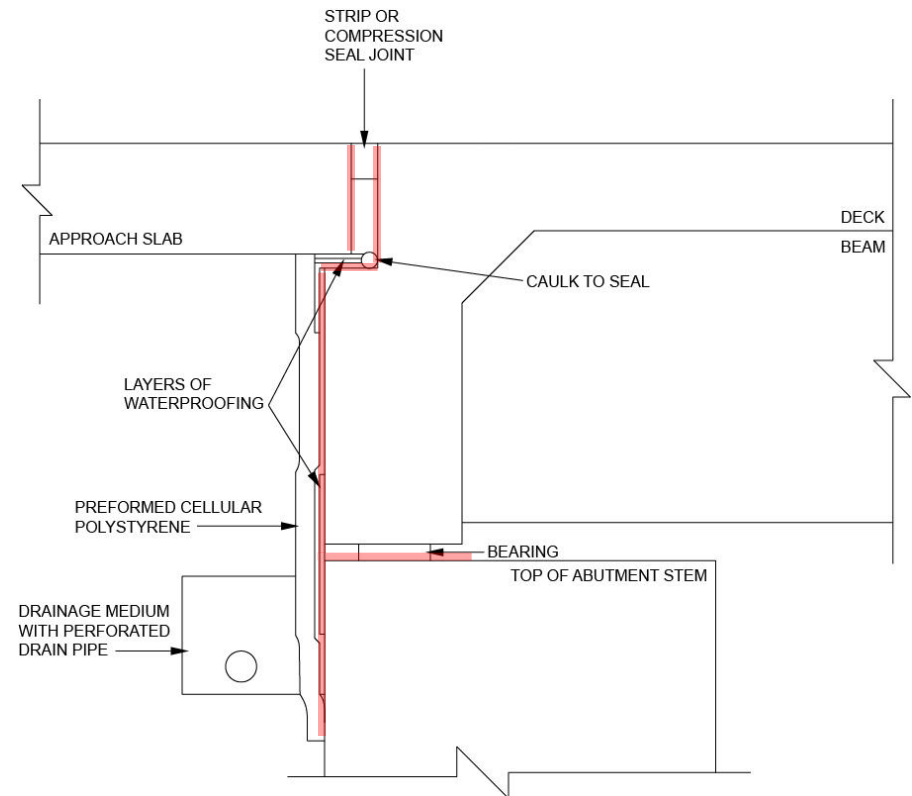
Integral Abutment

- No expansion joint between abutment and superstructure
- Main areas of concern:
 - Leakage at construction joints in:
 - Abutment stem
 - Abutment-Deck interface (Due to poor construction quality)
 - Stress-induced cracks due to inadequate design (difficult to design stress redistribution)



Parapet Abutment

- Expansion joint between abutment and superstructure (on at least one side of bridge)
- Main areas of concern:
 - Leakage at abutment – superstructure interface
 - Leakage at construction joints (Due to poor construction quality)
- Note: Old-new concrete interface in rehab projects also area of concern





Comparison of Integral and Parapet type abutments

- Integral abutments have restrictions such as bridge span, soil strata, loading restrictions, etc. and are more challenging to design
- Parapet abutments are more versatile in the type of retaining wall used, has almost no restrictions and are easy to design
- Integral abutments are cheaper to build, maintain and have a long life
- Parapet abutments are more expensive to build, maintain and have to be repaired or rehabilitated at least once in the lifespan of the structure



Part 3

**REVIEW OF DESIGNS FROM
DIFFERENT DEPARTMENTS
OF TRANSPORTATION**

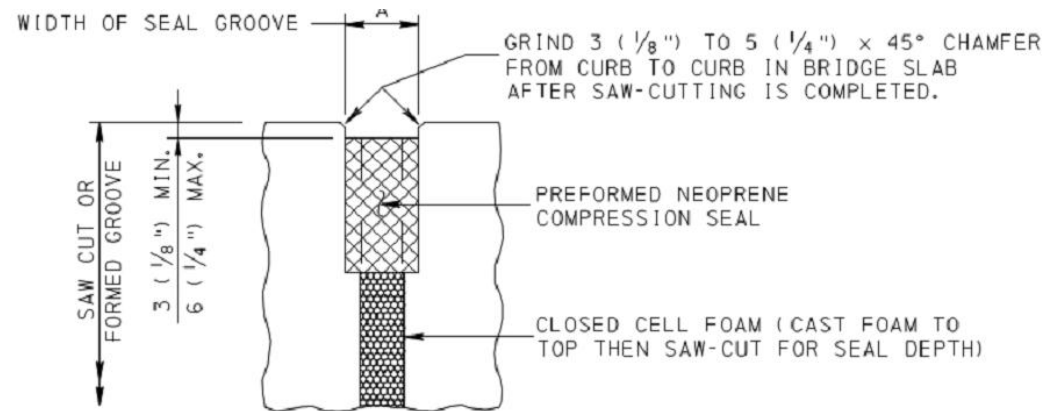


State DOT practice

- States selected based on similar weather pattern to Pittsburgh and surrounding areas
- Initial comparison of waterproofing design and inspection procedure from Massachusetts, Minnesota and Ohio
- Further comparison of waterproofing design from New York and Illinois

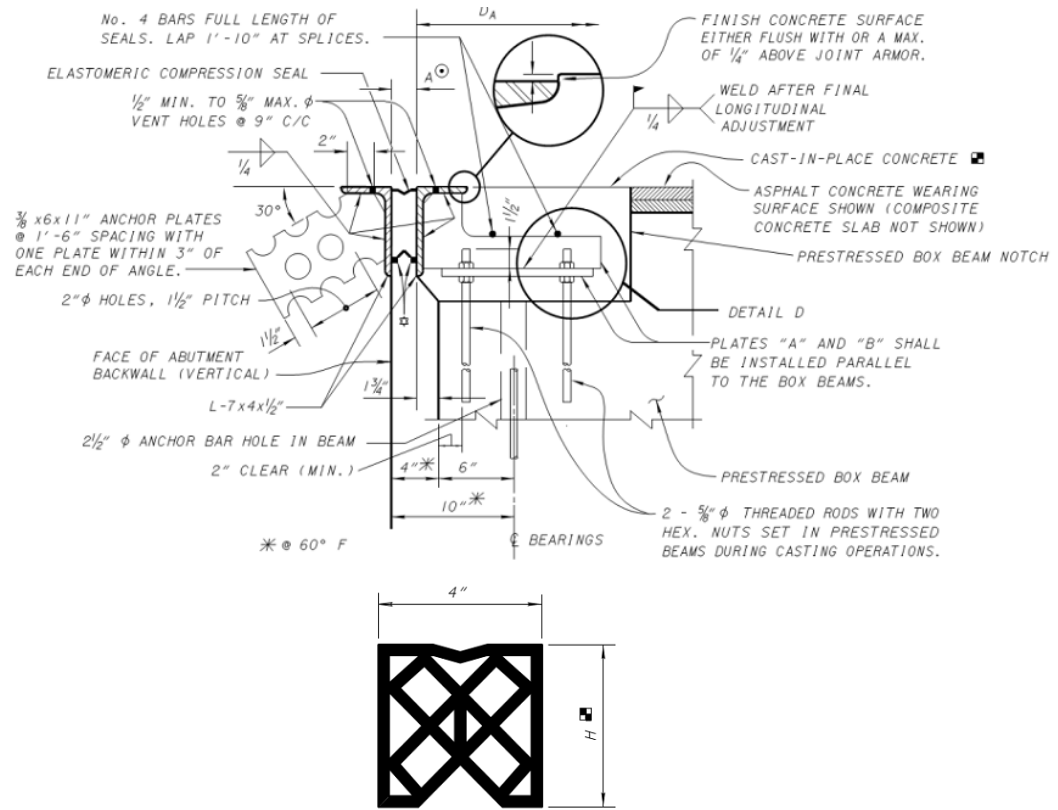
Compression Seal Joint - PennDOT

- No skew angle limit
- Fabricator provides joint opening size and compression seal design
- Only unarmored type used



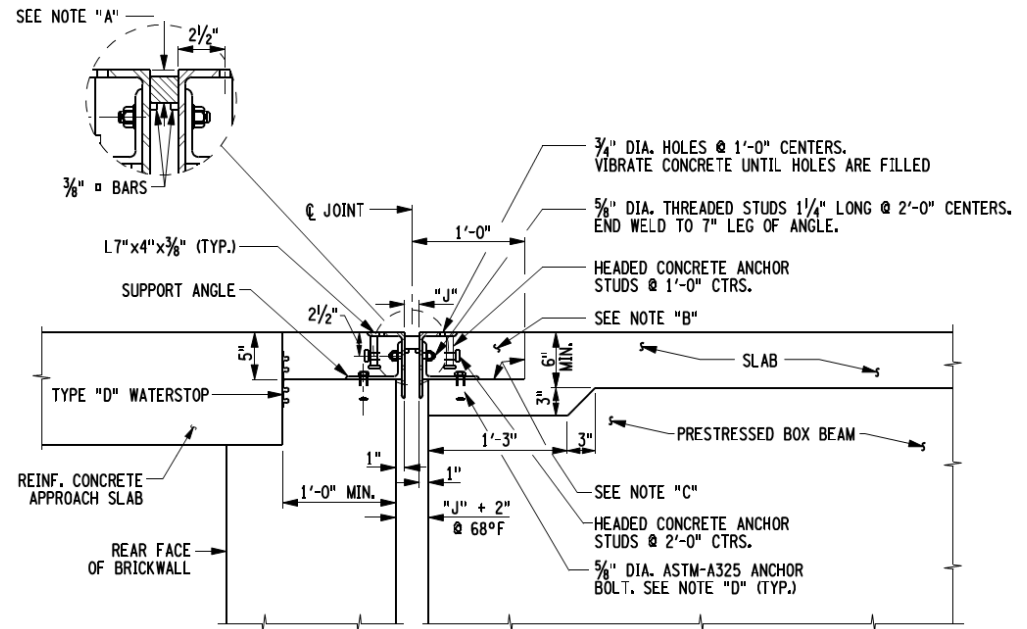
Compression Seal Joint - ODOT

- Skew angle limit – 15°
- Specific about joint opening size (during installation) and compression seal design
- Only armored type used



Compression Seal Joint - NYSDOT

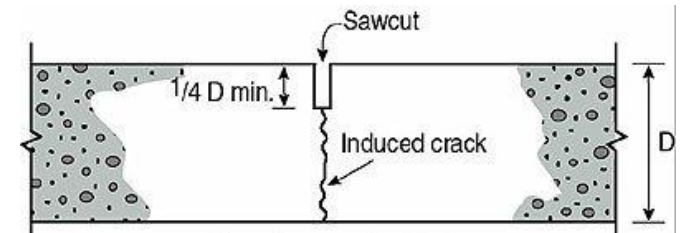
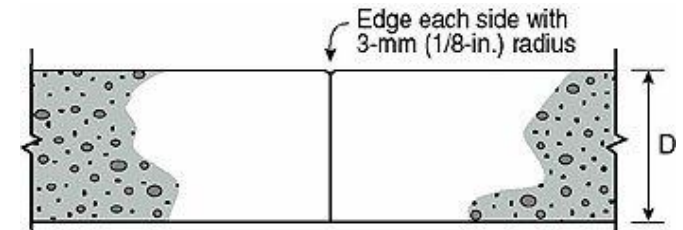
- Skew angle limit – 45°
- Fabricator provides joint opening size and compression seal design
- Only armored type used





Compression Seal Joint – Summary

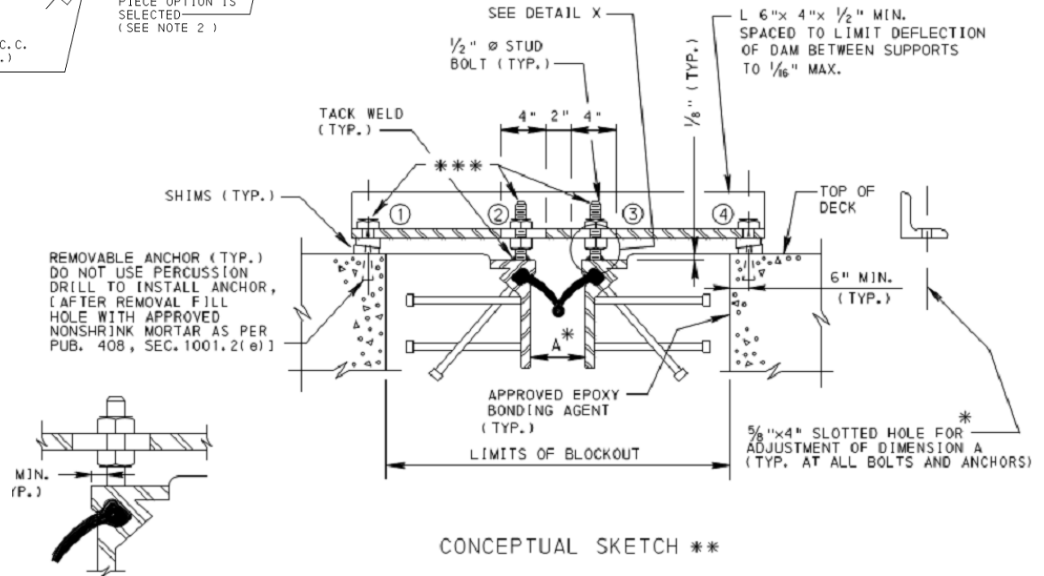
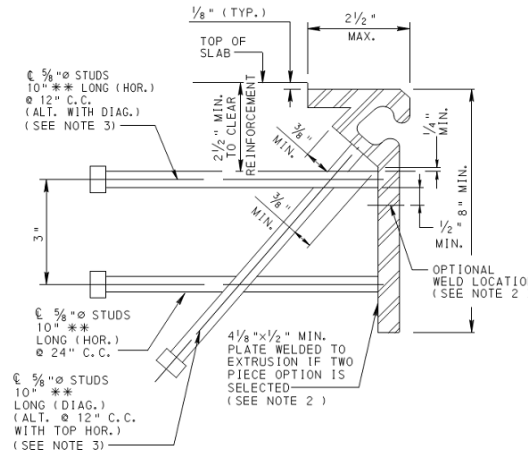
- MnDOT specifies min. 5 cells in compression seal; Only used in contraction joints
- Only PennDOT does not specify skew angle
- PennDOT only uses unarmored joint; ODOT and NYSDOT only use armored joint





Strip Seal Joint - PennDOT

- No skew angle limit
- Fabricator to provide opening size and strip seal design



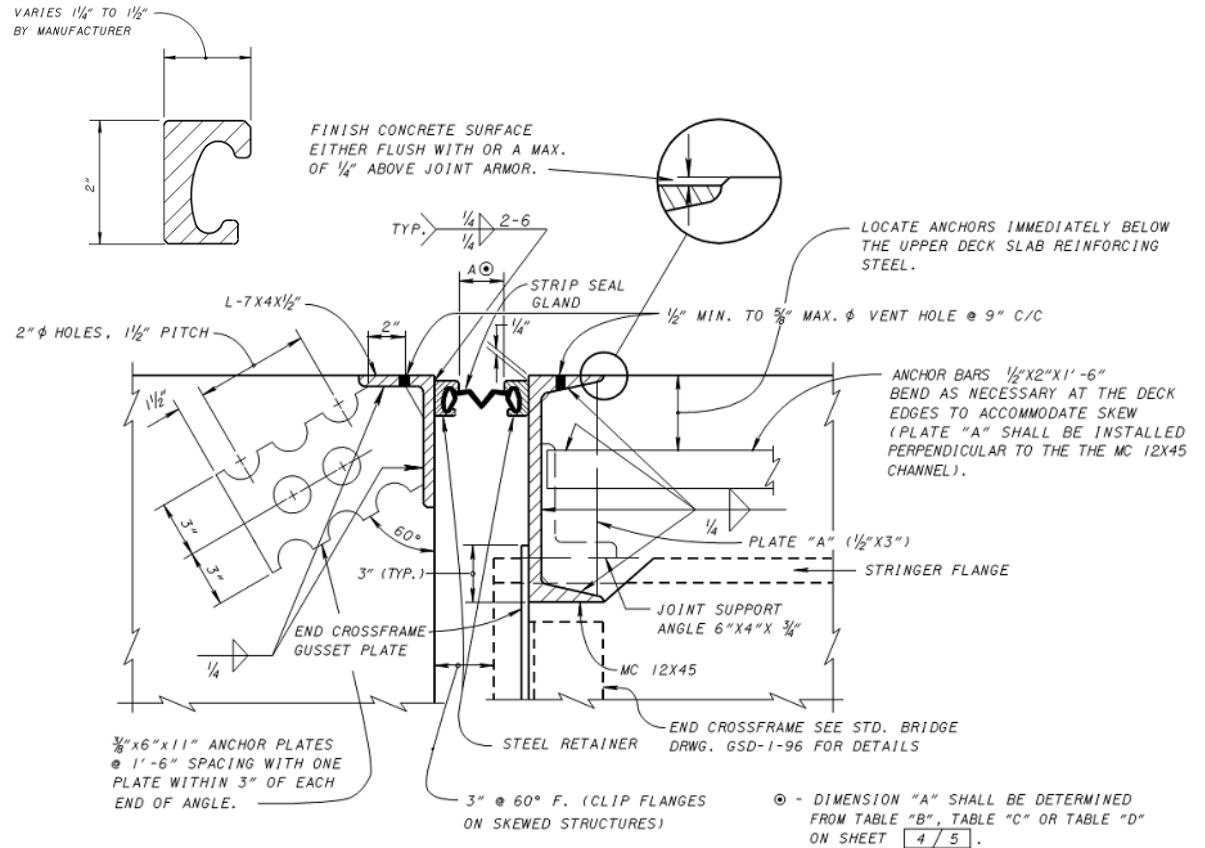
DETAIL X

JOINT INSTALLATION SCHEME

CONCEPTUAL SKETCH **

Strip Seal Joint - ODOT

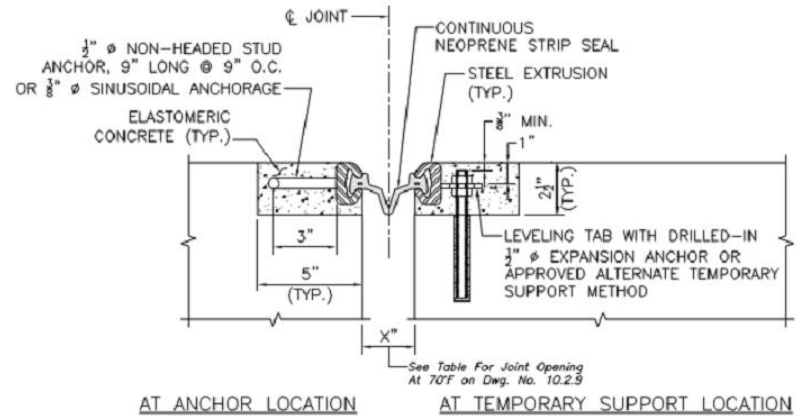
- Skew angle limit – 60°
- Specific about joint opening size (during installation) and strip seal design





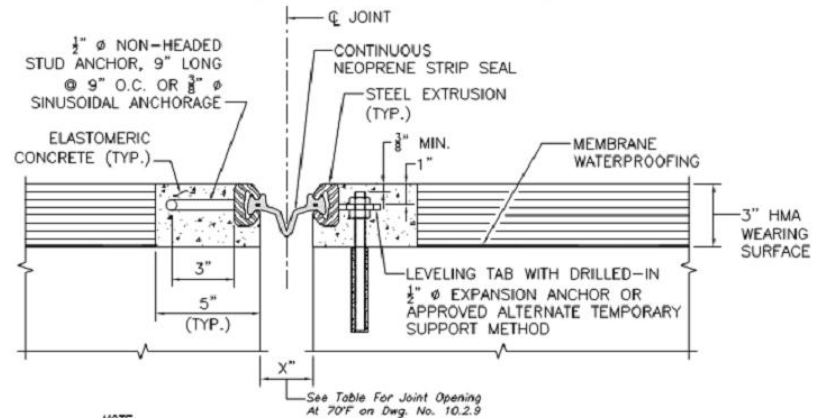
Strip Seal Joint - MassDOT

- No skew angle limit
- Fabricator to provide opening size and strip seal design



AT ANCHOR LOCATION AT TEMPORARY SUPPORT LOCATION

SECTION 1
SCALE: 3" = 1'-0"
(EXPOSED CONCRETE DECKS)



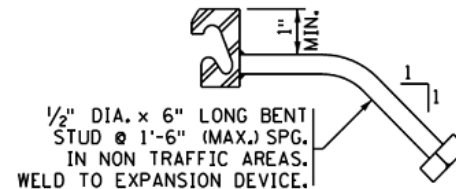
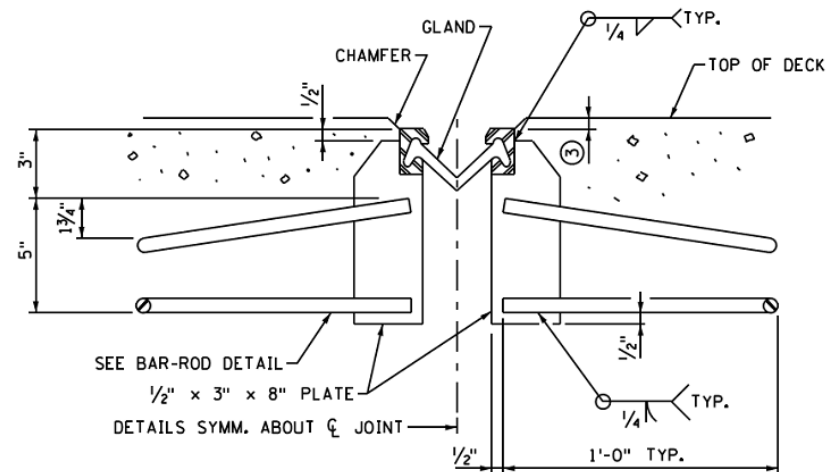
NOTE:
This detail must be used with deck drains. See Dwg. No. 7.3.1 for details.
AT ANCHOR LOCATION AT TEMPORARY SUPPORT LOCATION

SECTION 1
SCALE: 3" = 1'-0"
(DECKS WITH HMA WEARING SURFACE)



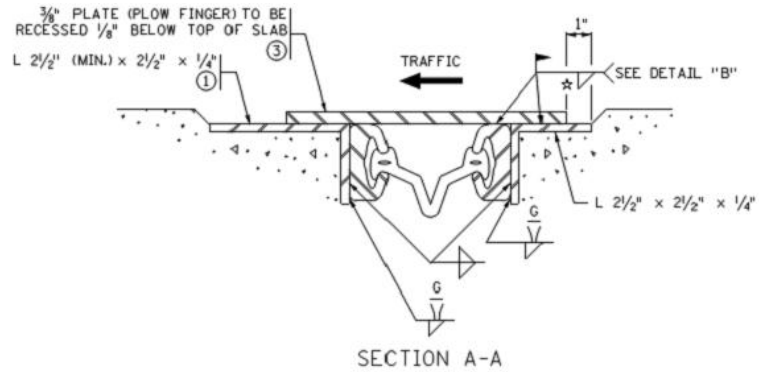
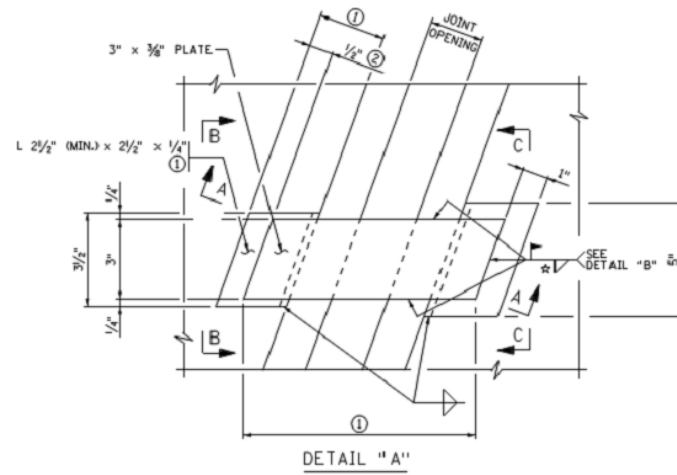
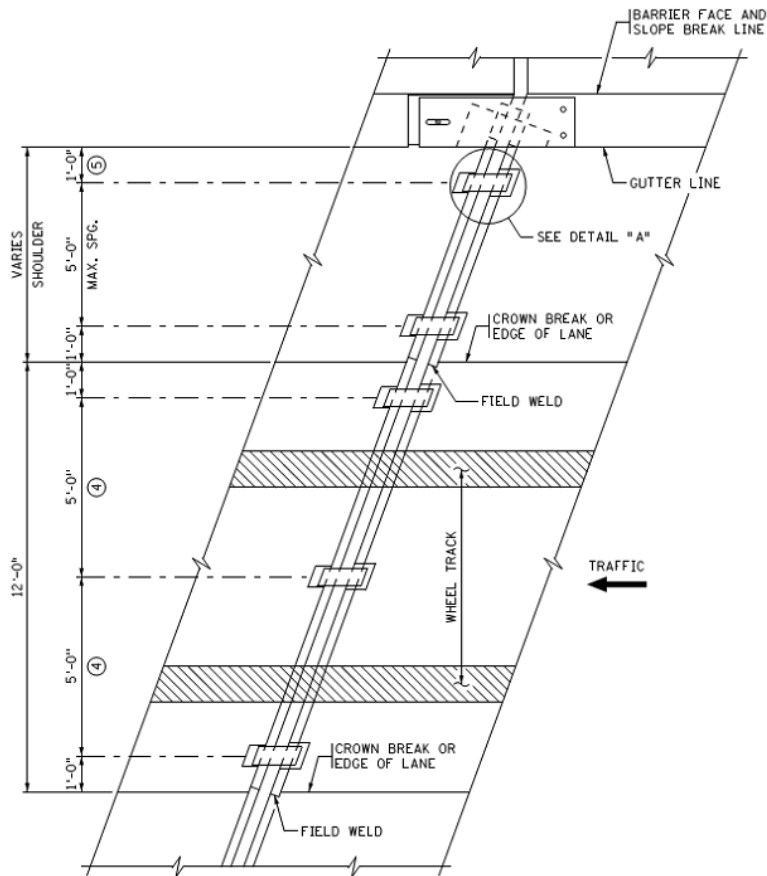
Strip Seal Joint - MnDOT

- No skew angle limit
- Fabricator to specify opening size
- Plow finger provided to protect against snow plows only in skew angles from 15° to 50°



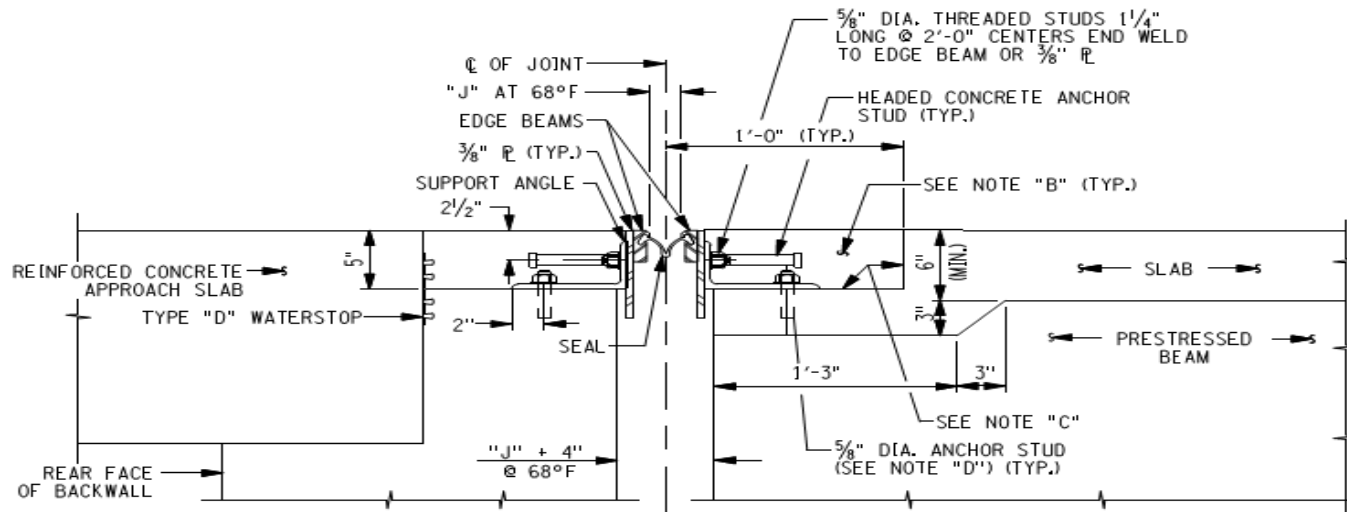


Strip Seal Joint - MnDOT





Strip Seal Joint - NYSDOT



- No armor on block-out
- Fabricator to provide opening size and strip seal design
- Span of deck restricted based on skew angle

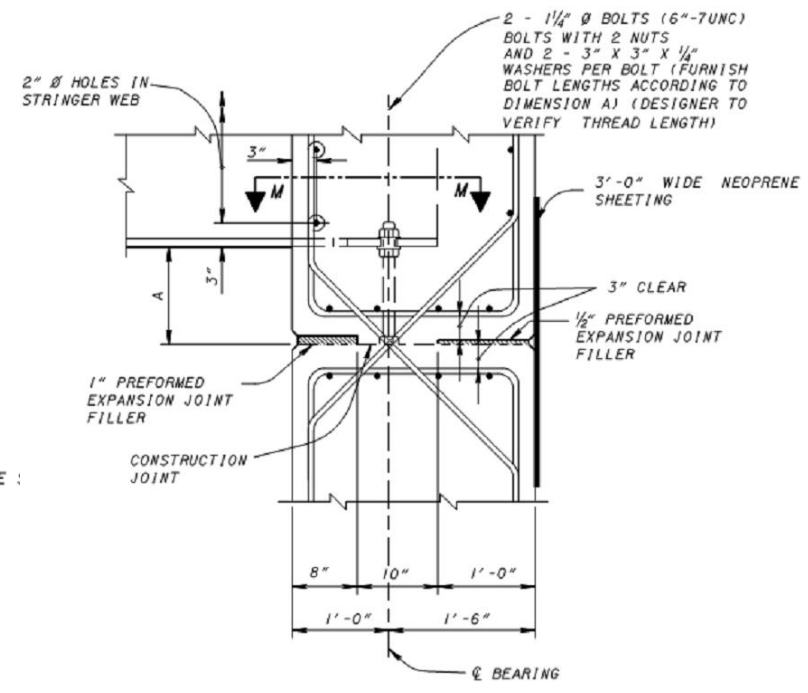
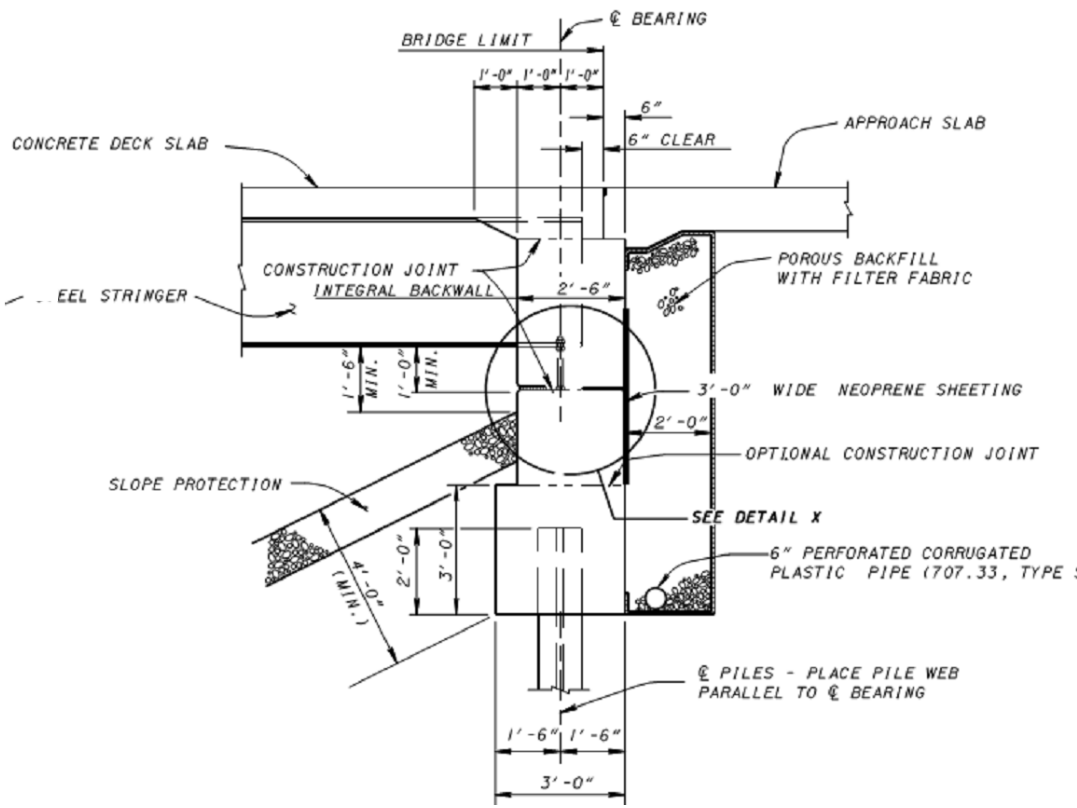


Strip Seal Joint - Summary

- Only ODOT limits skew angle (60°)
- MnDOT uses plow finger
- Only ODOT and MnDOT specify joint opening size during installation
- Except for MassDOT and ODOT, all other states use the same concrete in the block-out and deck; MassDOT specifies Elastomeric concrete and ODOT specifies the strength to be 4.5 ksi for the block-out

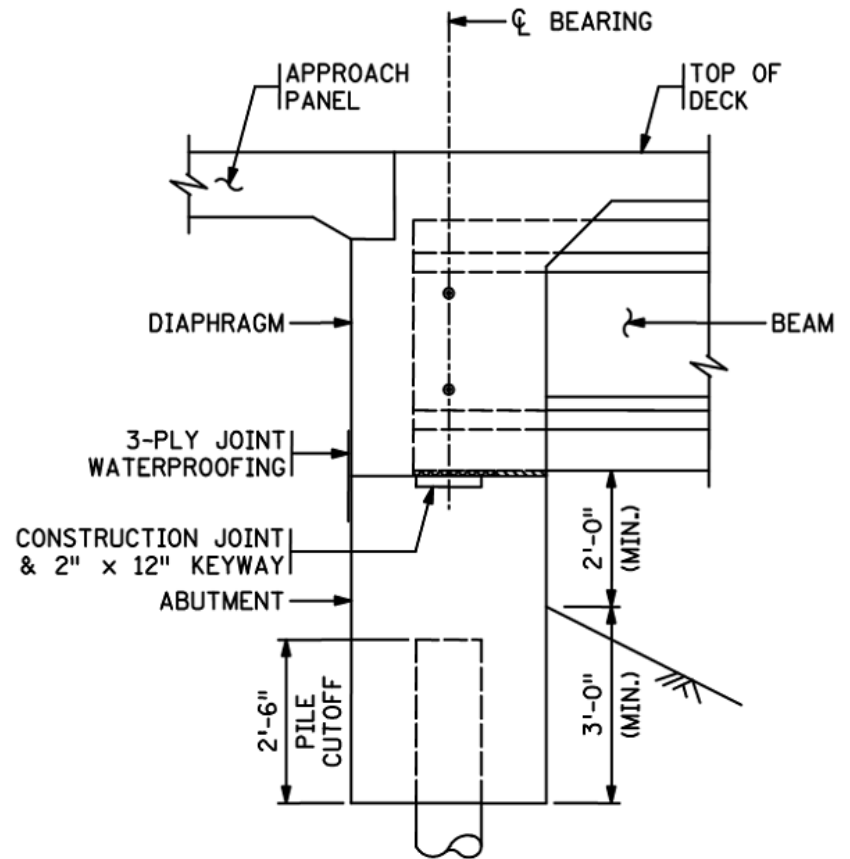
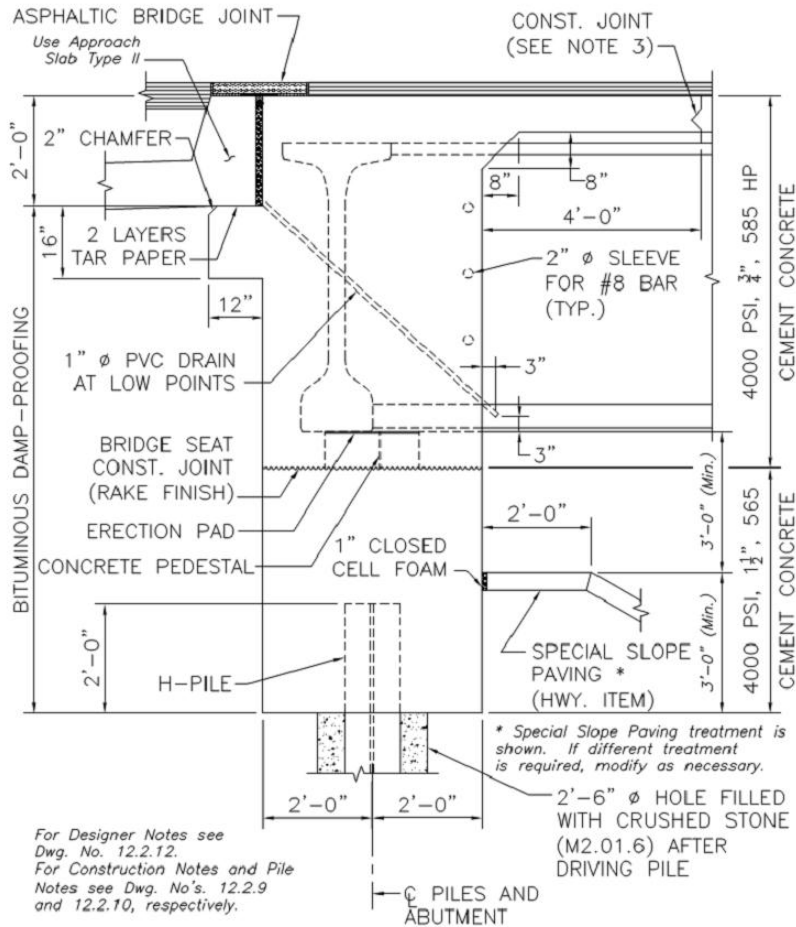


Integral Abutment - ODOT





Integral Abutment – MassDOT & MnDOT



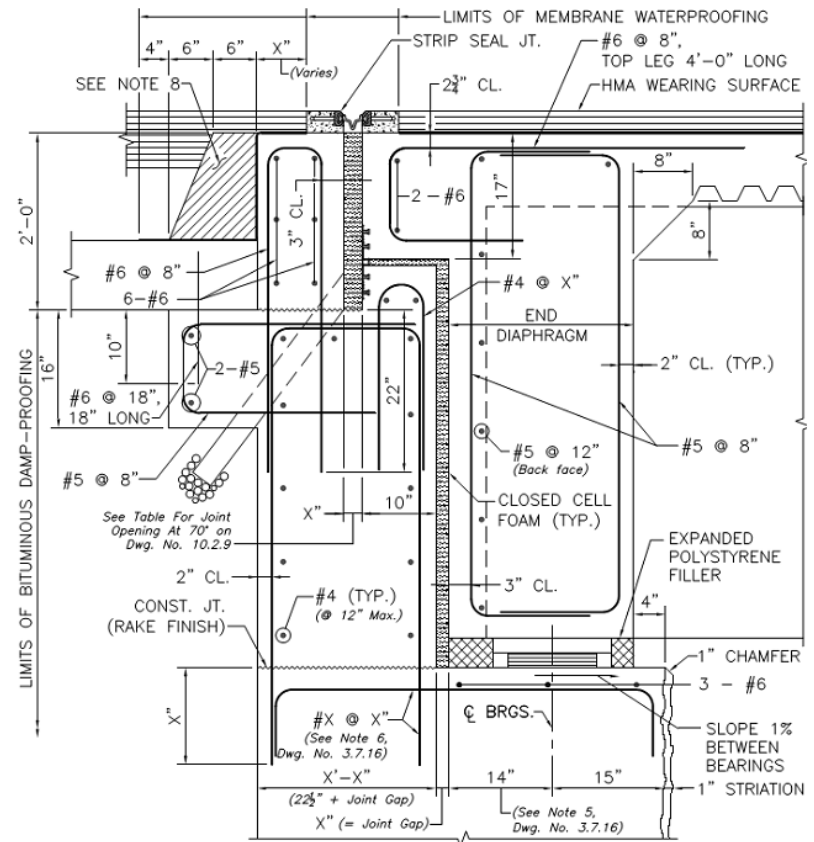
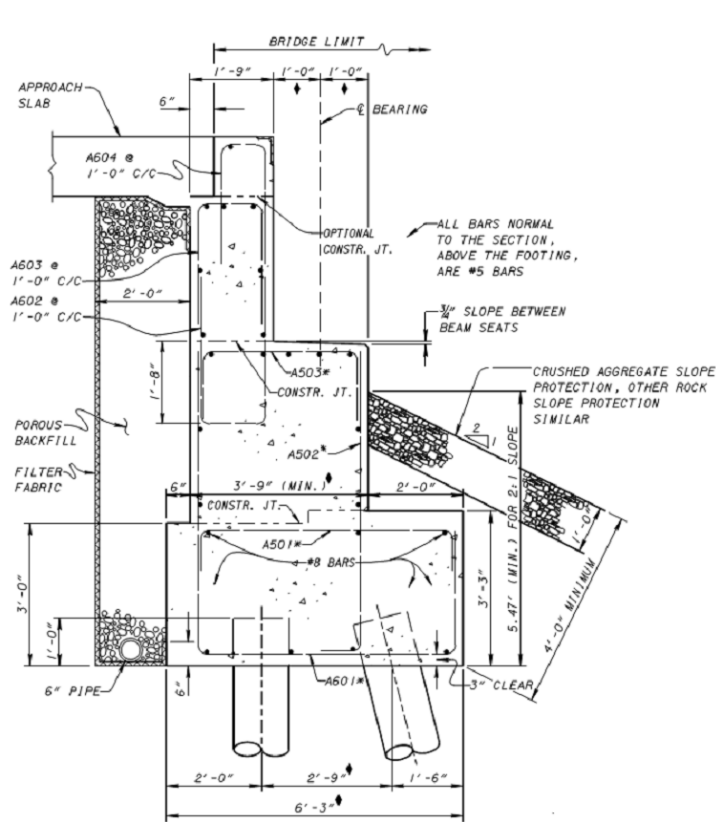


Integral Abutment – Summary

- Only MassDOT provides a coated water proofing. Other DOTs use (some sort of) waterproofing sheet
- PennDOT is least specific in waterproofing design detail

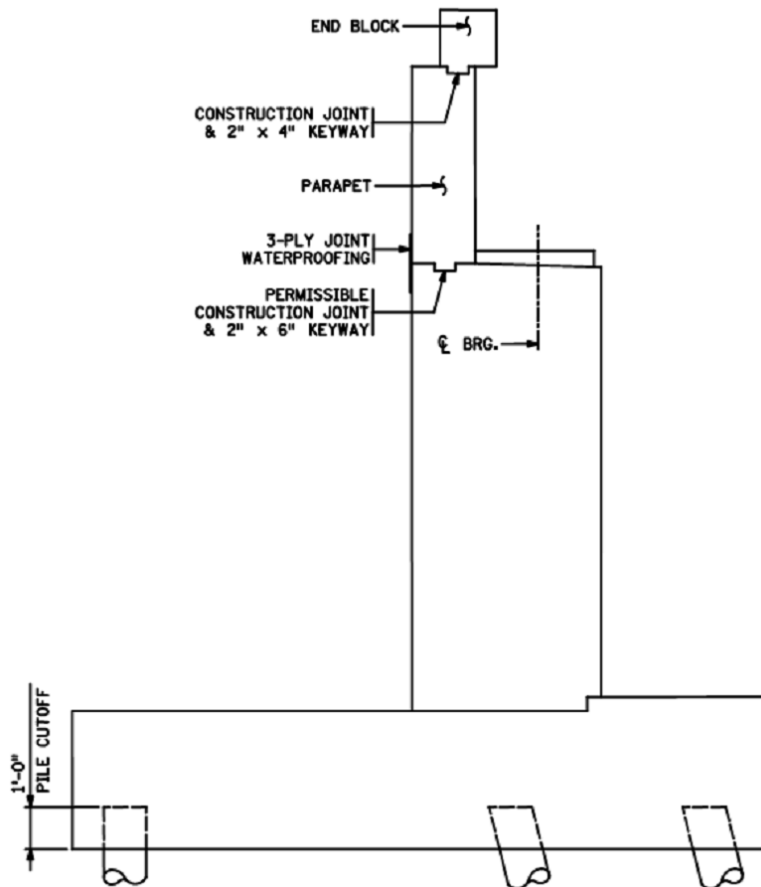


Parapet Abutment – ODOT & MassDOT





Parapet Abutment - MnDOT



- MassDOT – min. 2” thick membrane waterproofing; Waterstop in construction joints
- MnDOT - Membrane waterproofing – rubberized asphalt integrally bonded to polyethylene sheeting



Parapet Abutment - Summary

- Only PennDOT prefers abutment without backwall
- MnDOT lacks detail in abutment design – Presumably left to the designer
- Only ODOT uses a full length drainage backfill with filter fabric and perforated drain pipe
- Construction joints differ: Stepped/Flat type in ODOT, Raked in MassDOT & Keyed in MnDOT



Part 4

FIELD VISITS



Introduction

- 5 bridges selected – no older than 5 years – leakage issues
- 25 ft. to 225 ft. in length; 45 – 90 degrees skew angle;
- 3 field visits per bridge:
 - First visit on a sunny day
 - Second visit two days after rain
 - Third visit on a rainy day
- In addition, a construction site was visited to investigate waterproofing implementation



Little Creek Road Bridge



- Built 2010
- Skew angle: 70°
- Deck: 2 spans – 105 ft. & 120 ft.
- Compression seal: 0.5 in. movement



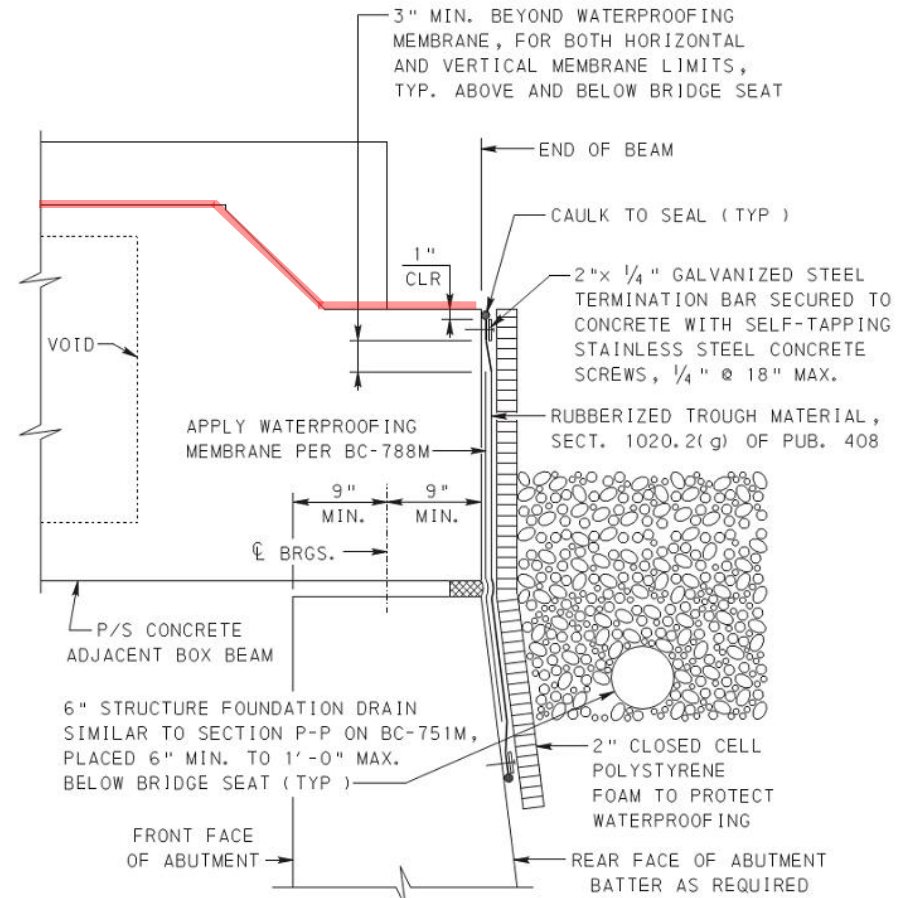
Little Creek Road Bridge

- Cracking & Erosion of joint edges
- Damage of silicone sealant at surface
- Leakage at construction joint at beam – deck interface





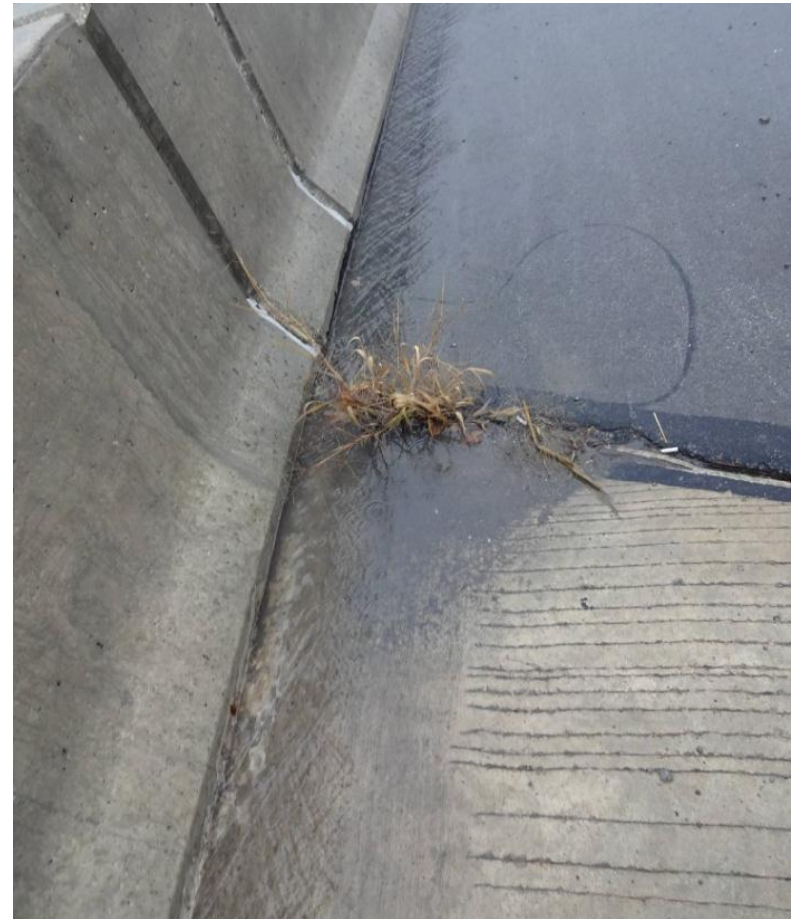
Little Creek Road Bridge





Little Creek Road Bridge

- Vegetation in joints
- Water seeping through joints
- Main cause of leakage – Expansion joint
- Leakage due to abutment possible as well





Prospect #1 Bridge

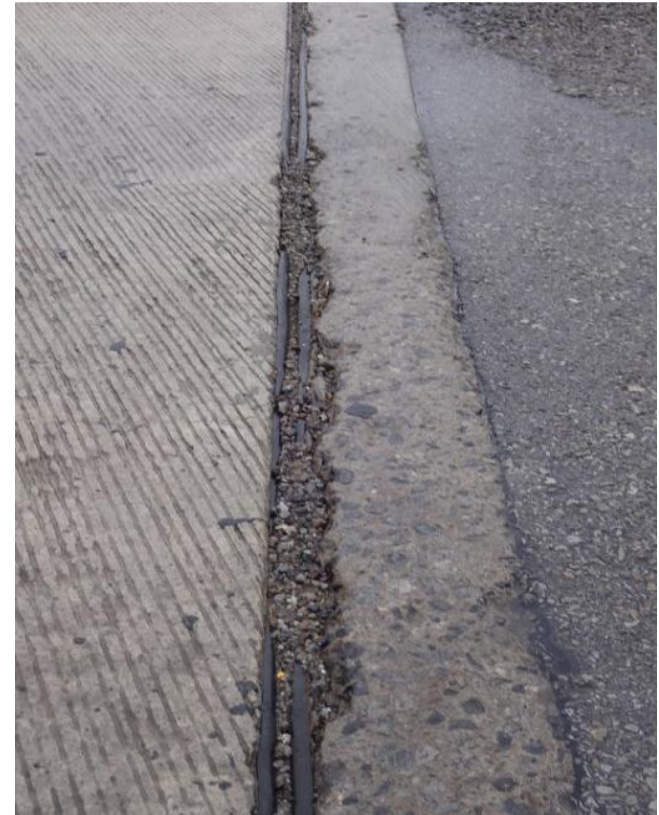


- Built 2009
- Skew angle: 70°
- Deck span: 104 ft.
- Compression seal: 1 in. movement (different from Little Creek Bridge)



Prospect #1 Bridge

- Debris accumulation on compression seal joint –
Due to recent road maintenance work –
Hinders drainage of water on deck
- Water staining on abutment – more severe near edges



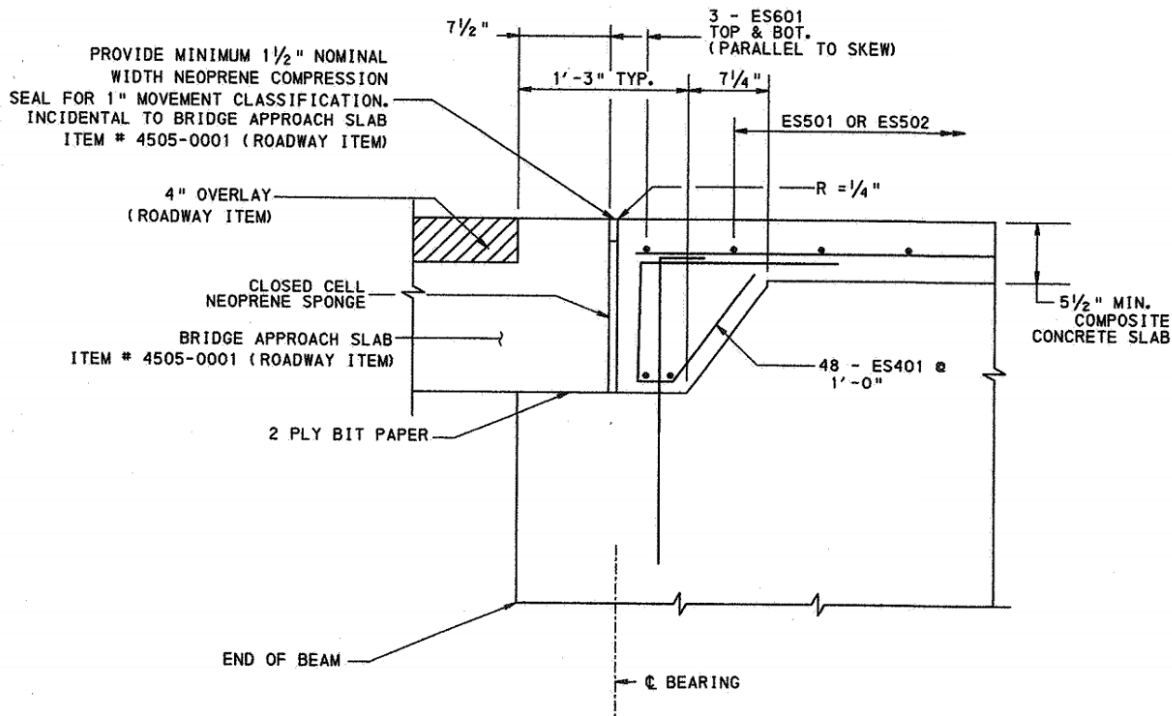


Prospect #1 Bridge





Prospect #1 Bridge





Prospect #1 Bridge

- Main cause:
 - Failure of concrete block-out – evident through leakage on edges
 - Leakage through box girder – both from expansion joint and backfill
- Debris accumulation exacerbates leakage issue
- Possible leakage of water through backwall
 - from sloped backfill – evident through weep hole leakage



SW of Boydstown Bridge

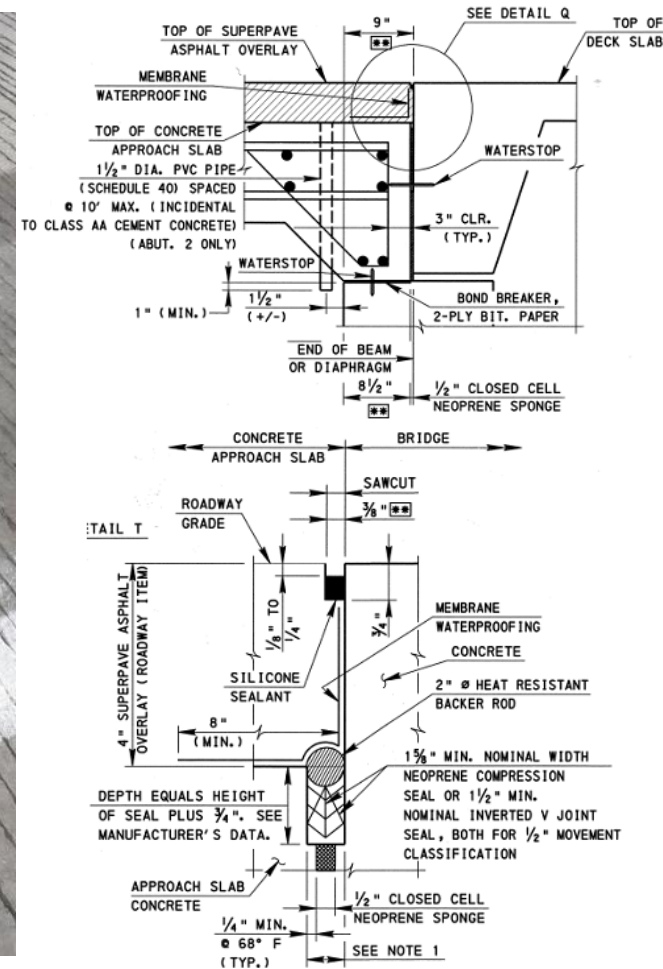


- Built 2012 – Rehab
- Skew angle: 60°
- Deck span: 25 ft.
- Compression Seal: 0.5 in. movement



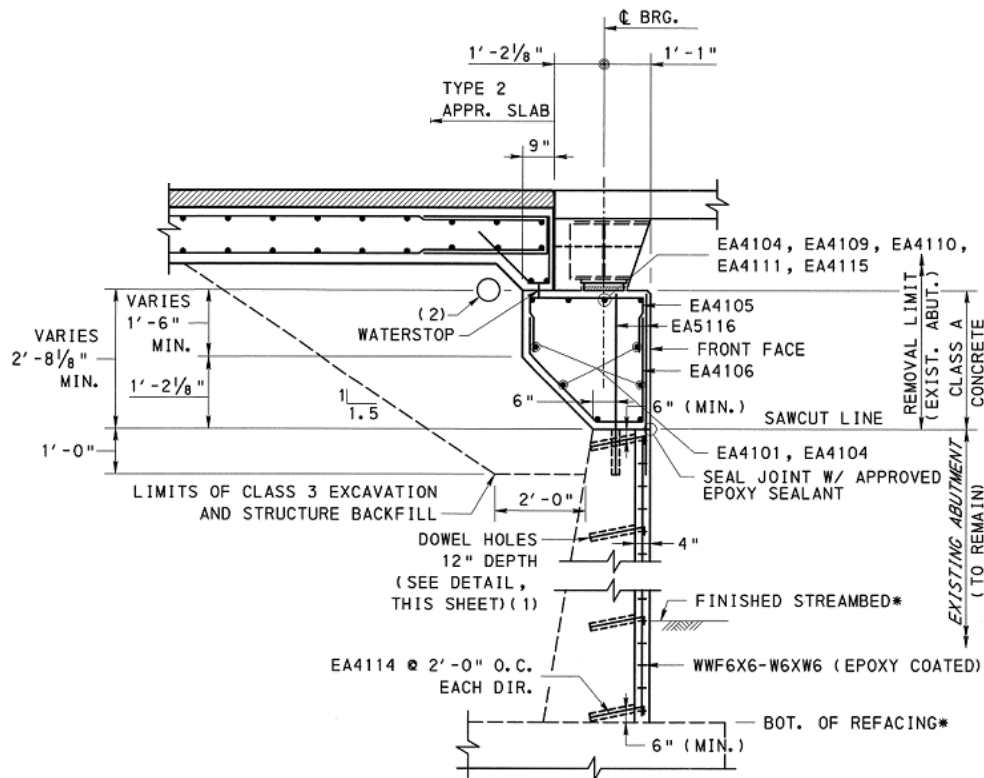
SW of Boydstown Bridge

- Initial signs of deterioration of silicone sealant in joint observed
- Leakage observed in old-new concrete interface on abutment
- Leakage at construction joint on abutment
- Deck slope insufficient to drain silt
- Silt found in weep holes





SW of Boydstown Bridge





SW of Boydstown Bridge





SW of Boydstown Bridge

- Main cause: Backfill through old-new concrete interface and construction joint
- Cracks due to differential shrinkage between new and old concrete – efflorescence exacerbating the situation
- No redundancy in abutment waterproofing
- Expansion joint not ruled out but highly unlikely – compression seal not visible – deck not sloped sufficiently



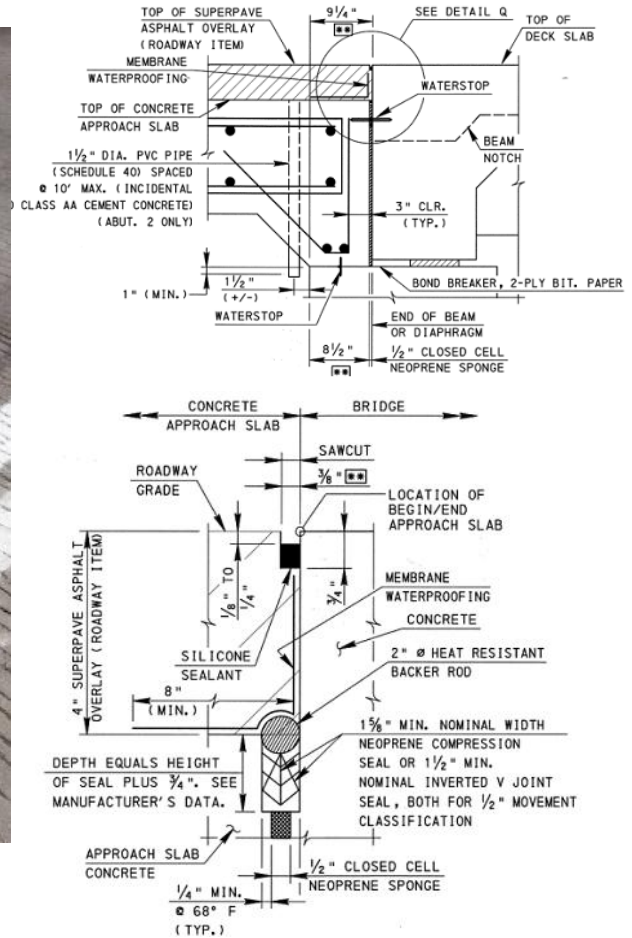
Little Connoquenessing Bridge



- Built 2012
- Skew angle: 45°
- Deck Span: 34 ft.
- Compression Seal: 0.5 in. movement

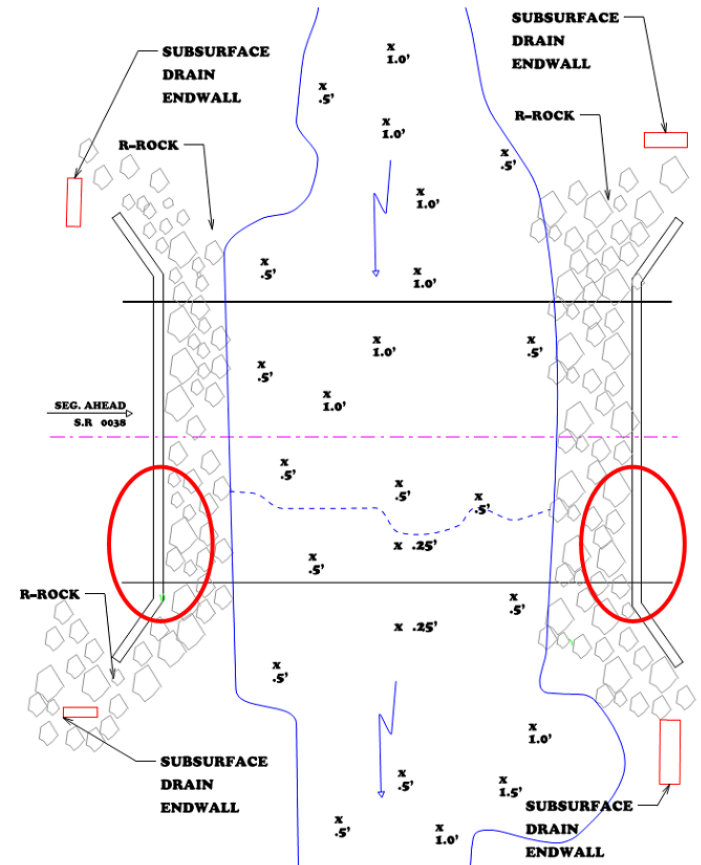
Little Connoquenessing Bridge

- Bridge still new; However showing initial signs of edge chipping
- Water staining on abutment stem more so on the east side





Little Connoquenessing Bridge





Little Connoquenessing Bridge

- Main Cause: Backfill through abutment seat
- Possible role of surrounding terrain
- Possible waterstop failure



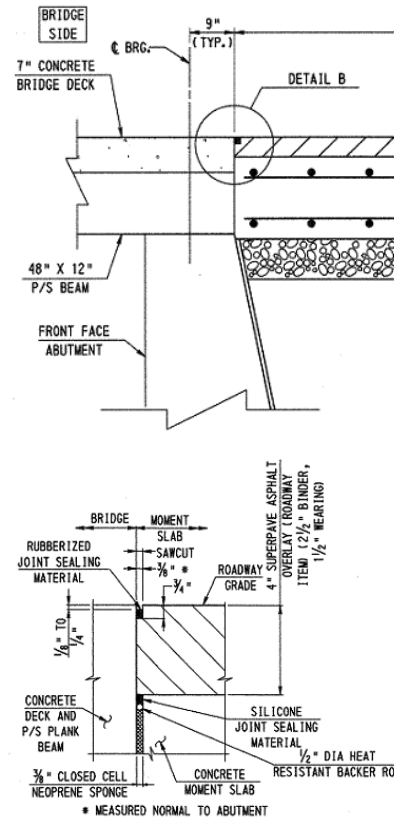
Beechton Bridge



- Built 2011 – Rehab
- Skew angle: 90°
- Deck Span: 31 ft.
- Rubberized joint sealing material

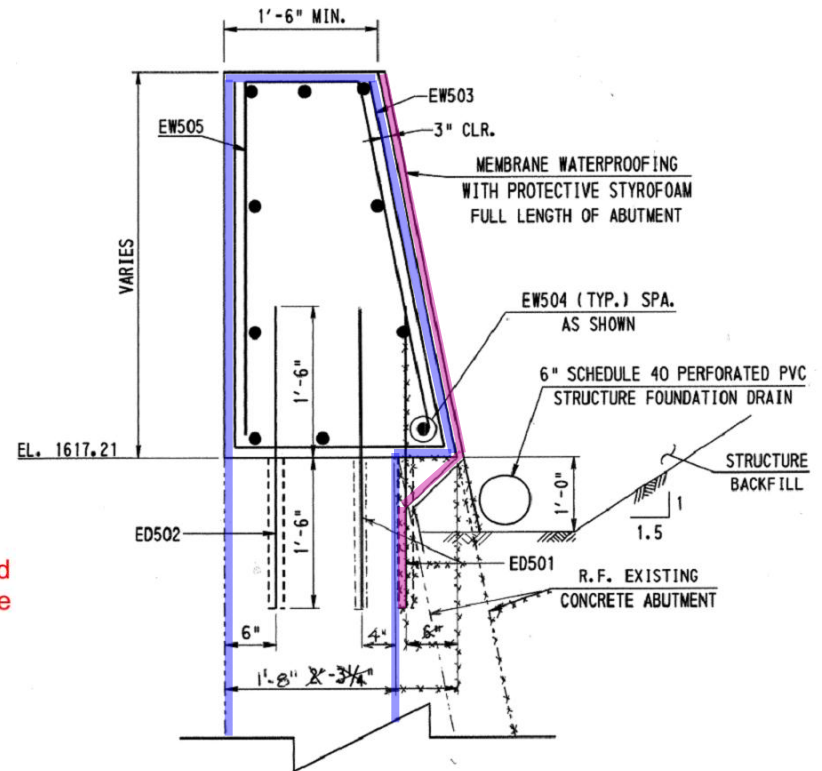
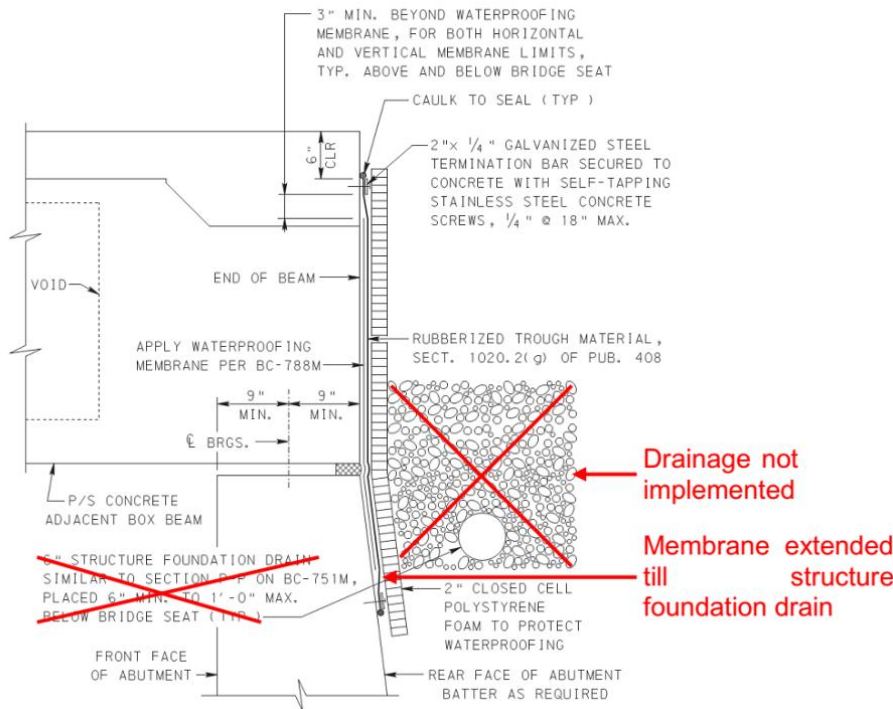
Beechton Bridge

- Severe leakage at old-new concrete interface on abutment
- Leakage at cracks propagating from the interface





Beechton Bridge





Beechton Bridge

- Main cause: Backfill through old-new concrete interface
- Waterproofing membrane bent – mostly failed
- Crack in new concrete due to differential shrinkage
- Only drainage at old-new concrete interface – not at bearing (explain observation of leakage at bearing seat)





Bridges from MassDOT, MnDOT, IDOT & NYSDOT

- No leakage in bridges from MassDOT; Only one integral abutment bridge shows leakage – mostly due to stress-induced cracks (stress redistribution is a design challenge)
- Strip seal preferred over compression seal in MnDOT; No leakage found even with cracks on abutment stem – mostly due to redundancy in waterproofing
- Strip seal reduces leakage in IDOT compared to compression seal
- Good performance of bridges with strip seal up to 15 years and compression seal up to 10 years (sizing is essential); Main issue with relatively early block-out failure compared to joint;



Part 5

RESEARCH CONDUCTED

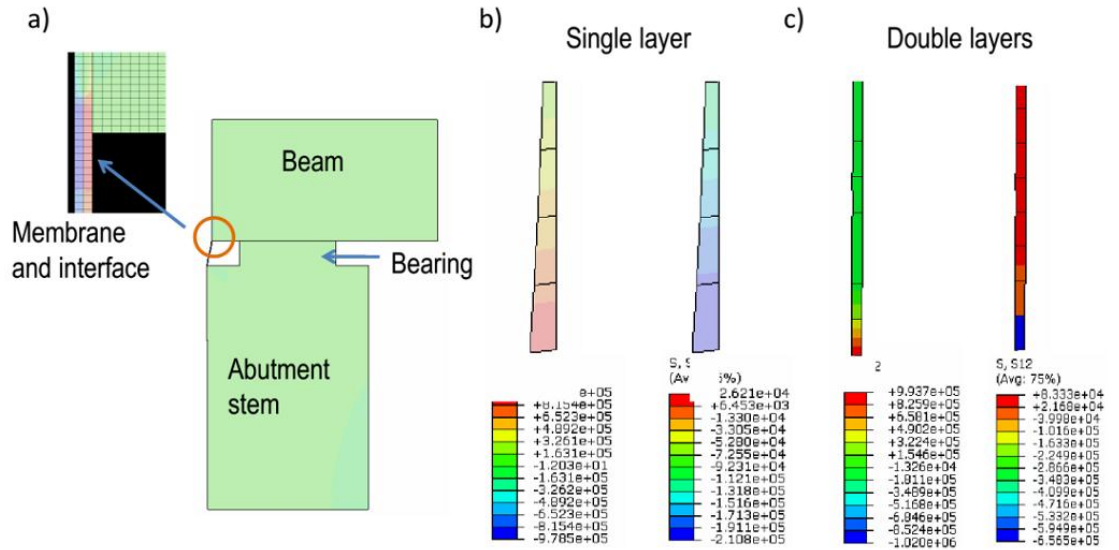


Research based Analysis

- Finite Element Modelling (FEM) of membrane peel off
- FEM of skew angle effect on compression seal joint
- FEM of traffic (impact) loading on steel extrusion (strip seal joint)
- Seal push out test of Strip seal (to check for safety against traffic loading with debris in seal)

Finite Element Modelling (FEM) of membrane peel-off

- To analyze effect of movement on waterproofing membrane at abutment seat
- Membrane extending 1 ft. on either side
- 100 ft. span – temp. variation of 104°F – 0.6 in. movement
- High stresses at membrane and interface – expected
- Double membrane layer – greatly increases stress concentration – possible adhesive failure





FEM of skew angle effect on compression seal joint

- To analyze effect of skew angle on compression seal joint
- 100 ft. span – temp. variation of 86°F – 0.4 in.
- Shear stress:
 - 20° – 75 psi
 - 45° – 139 psi
- Reduced safety margin at higher angles



Skew angle = 0 degree



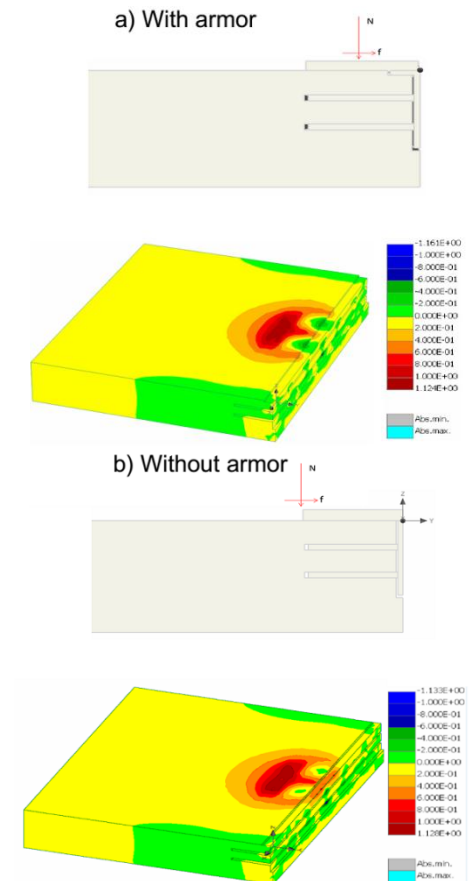
Skew angle = 20 degrees



Skew angle = 45 degrees

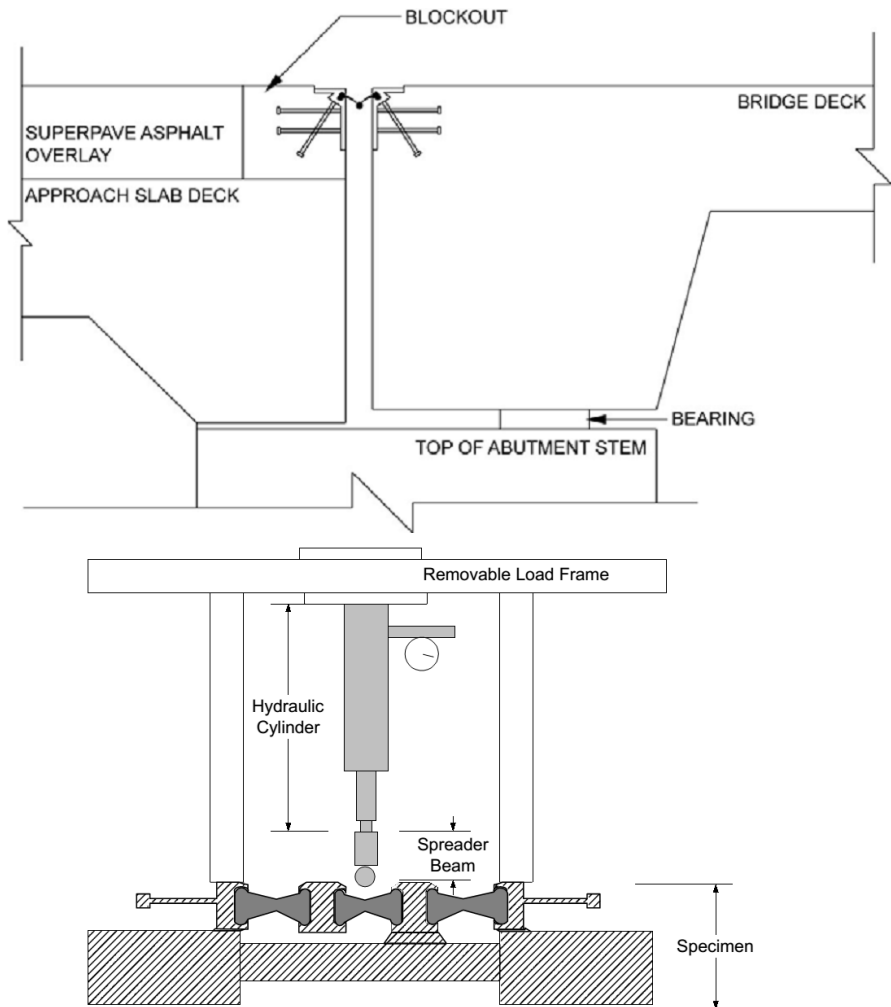
FEM of traffic (impact) loading on steel extrusion (strip seal joint)

- To analyze effect of traffic loading on anchorage stud of steel extrusion – with and without armor
- Max. stress in stud: 2118 psi
- Max. stress in extrusion: 1336 psi
- Max. block-out tensile stress: 163 psi
- Anchorage has very high safety margin when compared to the strip/compression seal it supports



Seal push out test of Strip seal

- Test to simulate loading of strip seal
- Reason is to test strength in case of traffic loading due to debris accumulation
- Modification of seal push out test by University of Minnesota (NCHRP Report 467)





Seal Push out test of Strip seal

- Sample from PennDOT approved manufacturer
- 3 ft. specimen; Spacing of 3 in. at top of the specimen
- Reaches around 4 in. of deflection and 2700 lb. of loading (1600 lb. reqd.)
- Great ductility and thus great tolerance to debris compared to compression seal





Part 6

CONCLUSIONS



Recommendations

- Based on study of design, field observations, analysis and testing, two kinds of recommendations are made:
 - Design based
 - Monitoring based



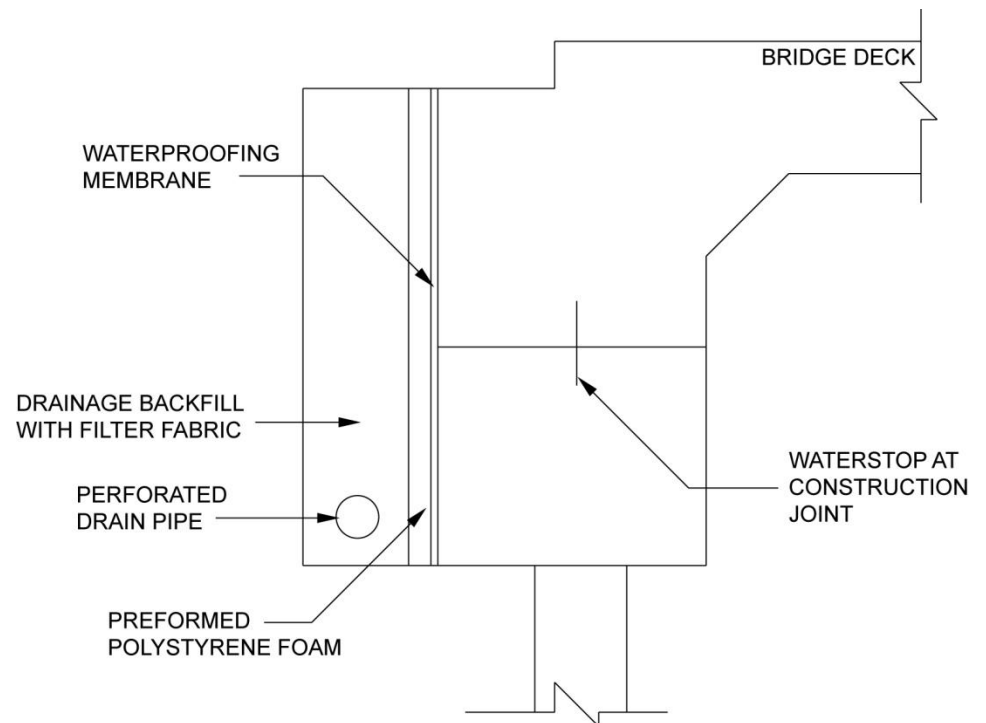
Design Recommendations

- Based on current design information
- Based on evaluation of design performance of other DOTs
- Based on research findings
- Six recommendations given based on broad categorization of project type and abutment type.
- Note: These are theoretical design guidelines; Actual design and other specifics need to be probed



Integral Abutment

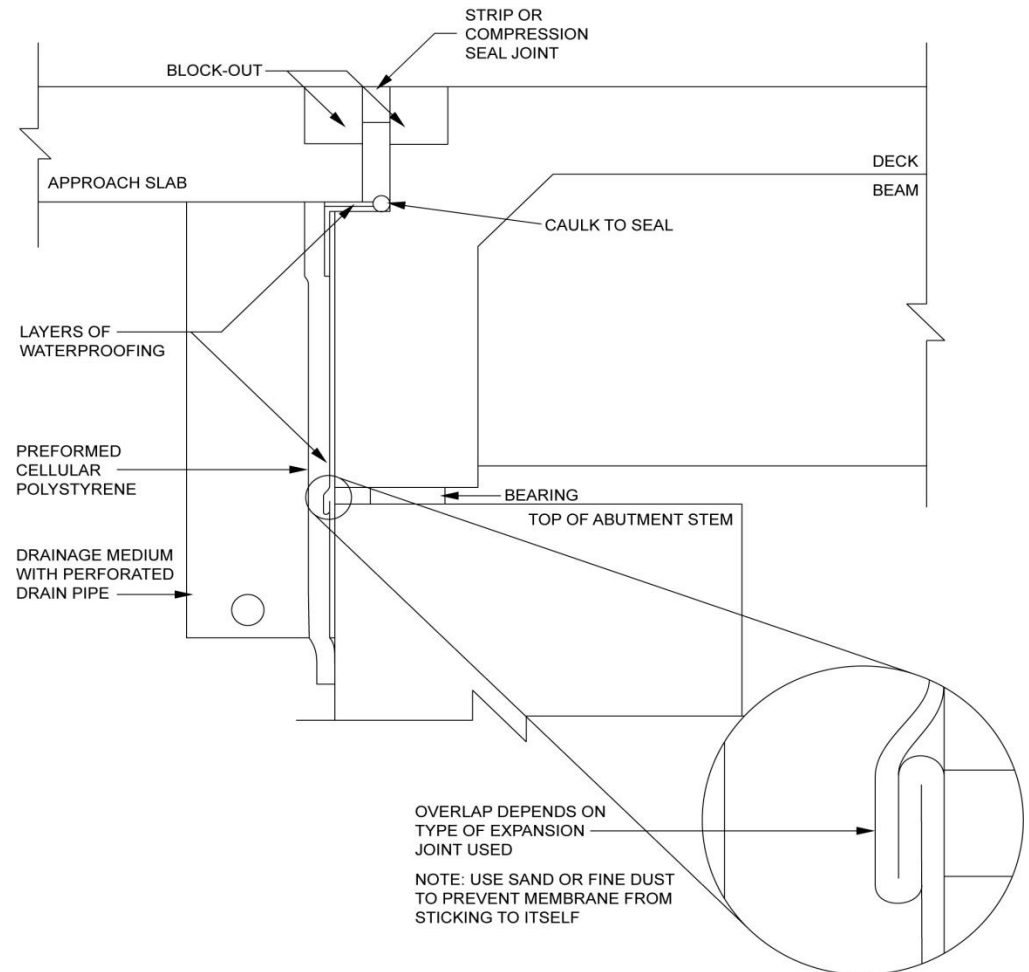
- Waterstop, membrane and full length drainage material for redundancy





Parapet Abutment – Without Backwall

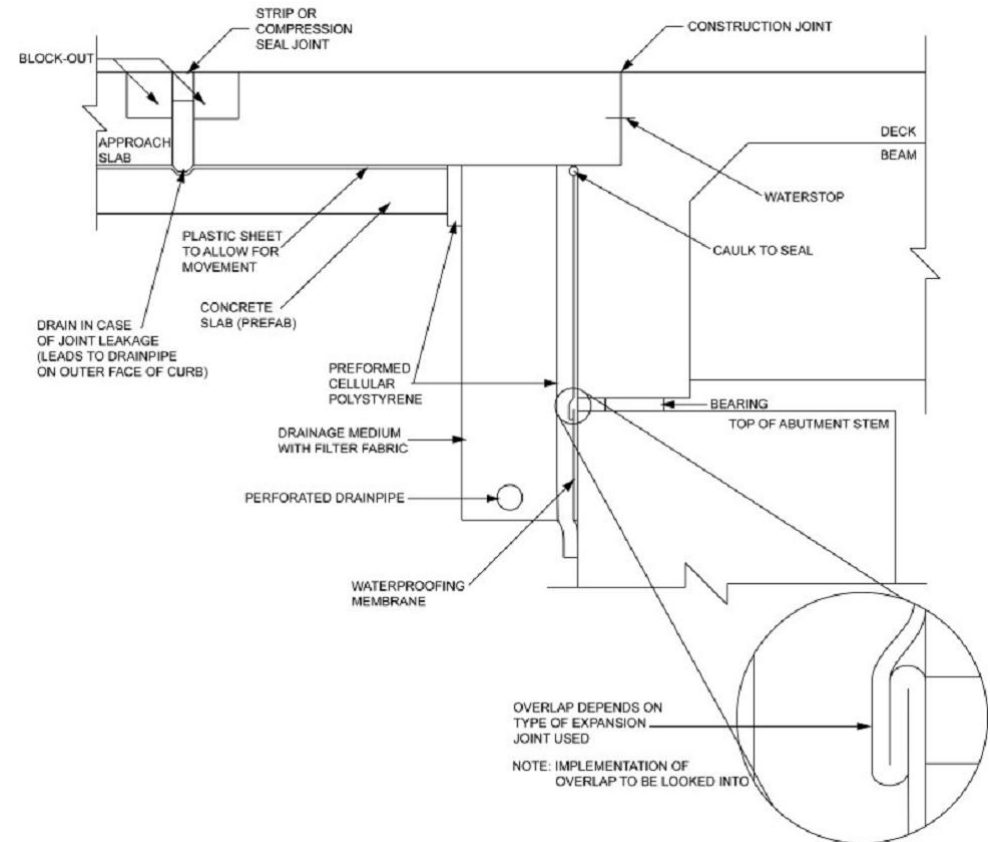
- Single layer of membrane at top of abutment stem area
- Joint is visually accessible
- Compression or Strip seal based on span and skew of deck
- Full height drainage layer with filter fabric





Parapet Abutment – Without Backwall

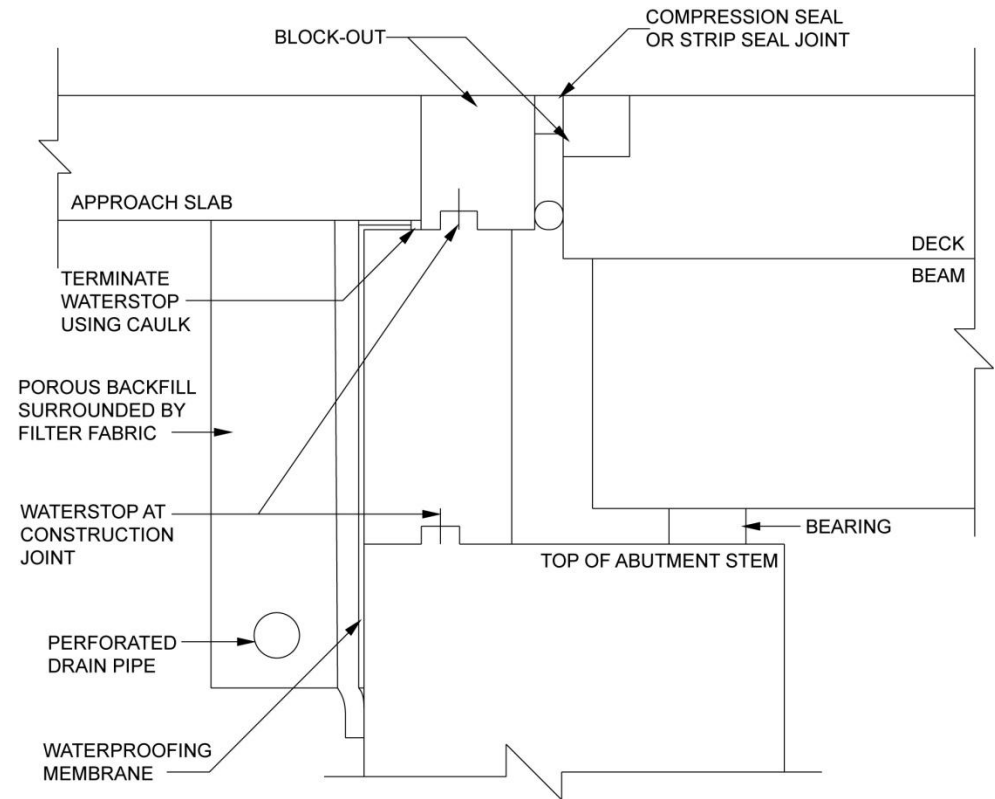
- Expansion Joint moved away from the abutment
- Joint is visually accessible
- Compression or Strip seal based on span and skew of deck
- Waterstop at all construction joints
- Full height drainage layer with filter fabric
- Requires higher levels of construction quality





Parapet Abutment – With Backwall

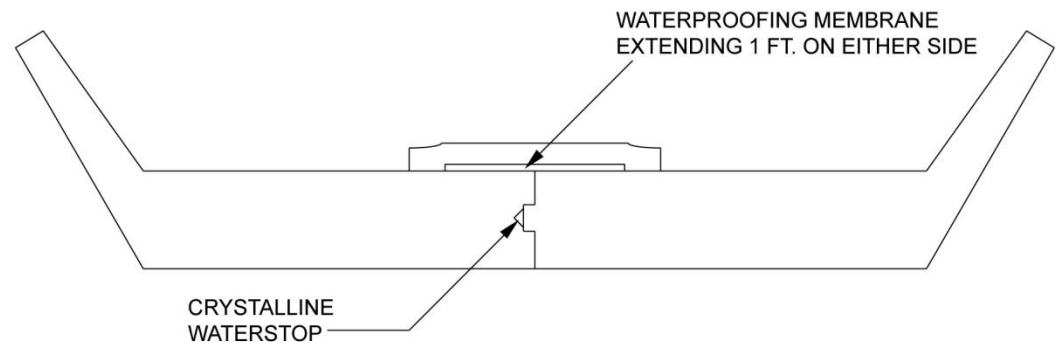
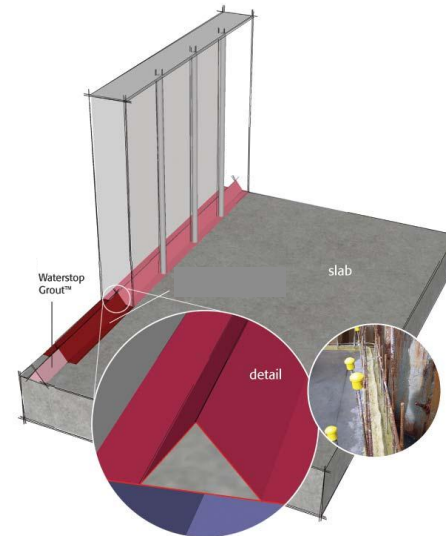
- Idea is to be able to predict source of leakage
- Joint is visually accessible
- Compression or Strip seal based on span and skew of deck
- Waterstop at all construction joints
- Full height drainage layer with filter fabric





Construction joint (in abutment)

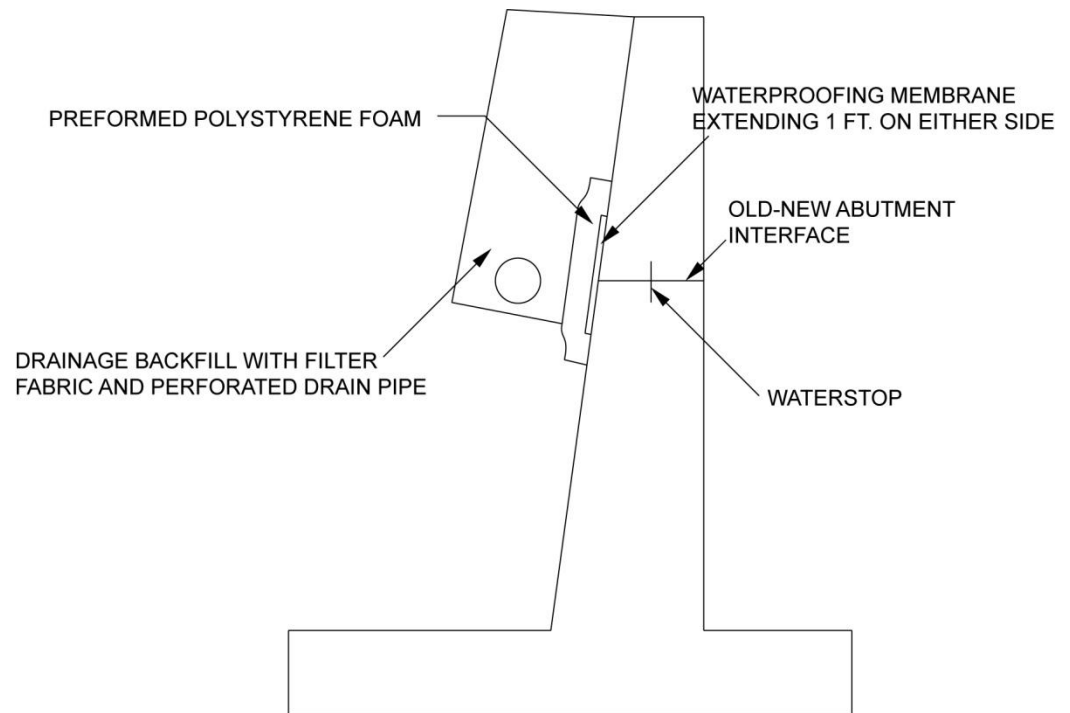
- PVC waterstop difficult to install – highly dependent on worker skill
- Crystalline waterstop suggested
- Waterproofing membrane suggested for full length (along with preformed cellular polystyrene) for redundancy





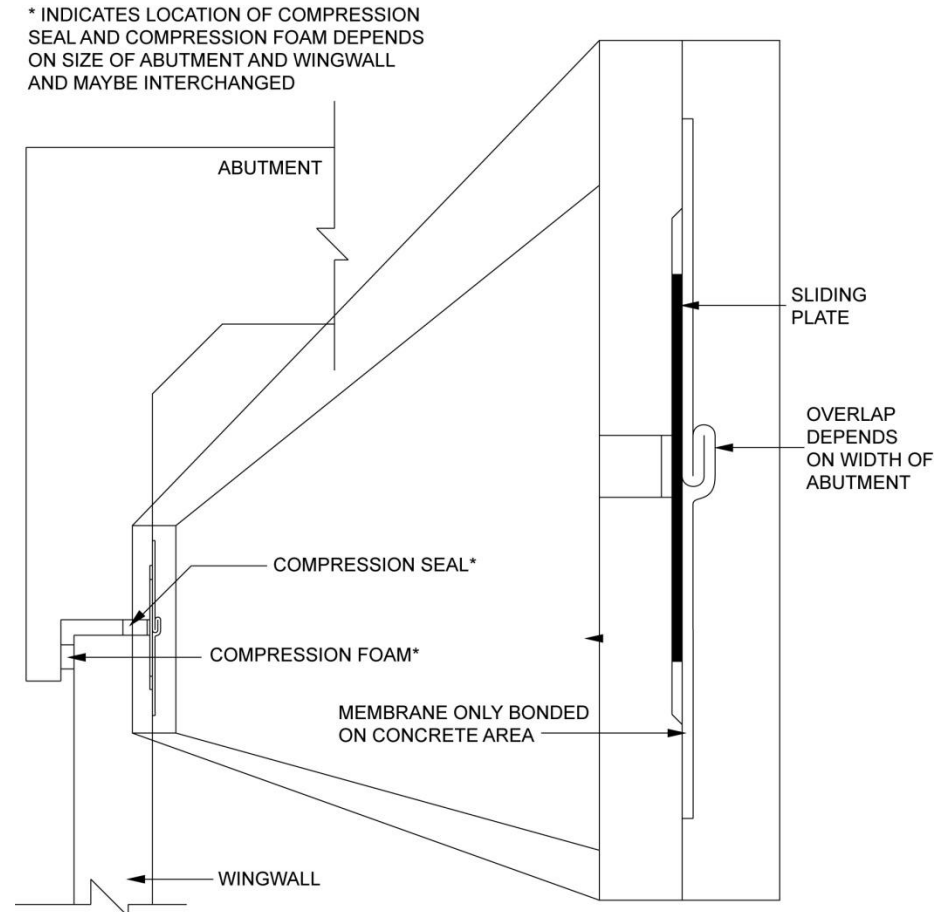
Old – New Concrete Interface

- Drainage backfill along with waterproofing membrane and waterstop for redundancy
- PVC waterstop installation to be further researched – Alternatively can use crystalline waterstop



Abutment – Wingwall Joint

- Primarily in integral abutments
- Waterproofing membrane with overlap
- Positioning of compression seal and foam depend on angle and size abutment and wingwall





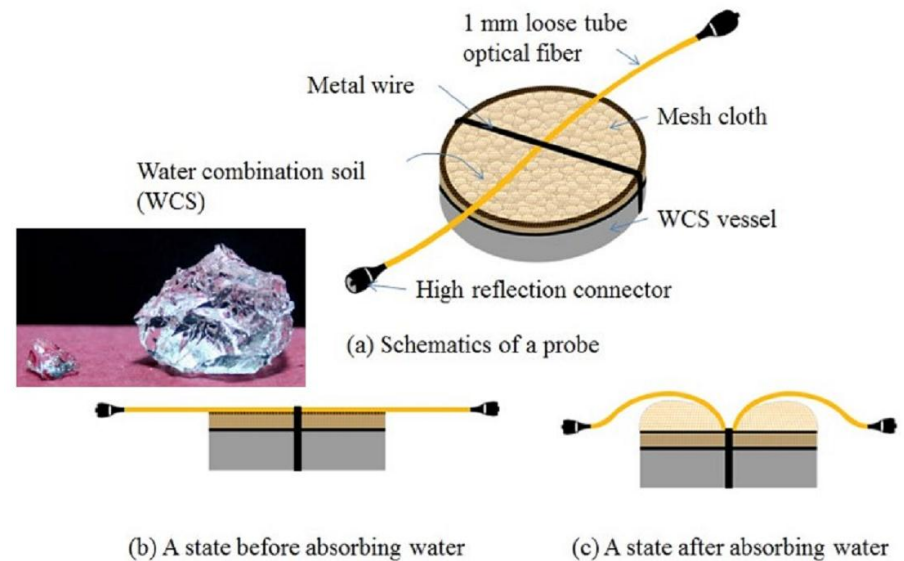
Monitoring Recommendations

- Based on issues encountered during field visits
- Based on currently available technology
- Fiber optics based testing
 - Optical Fiber Sensor
 - Evanescent Field Fiber Loop Ringdown (EF-FLRD)
- Strain gauge monitoring



Optical Fiber Sensor

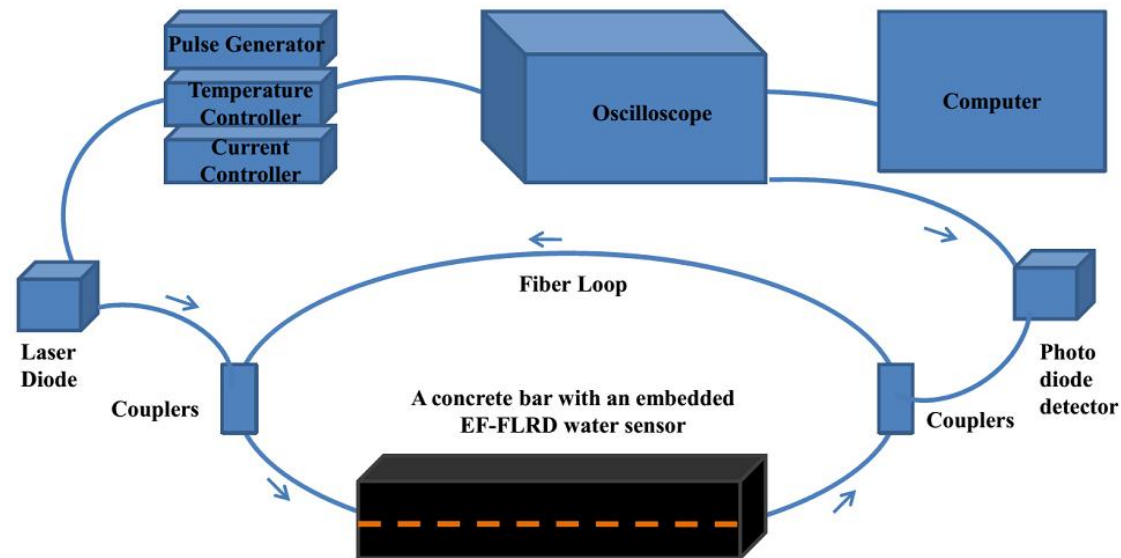
- Based on change of volume of a water absorption material
- One time use to detect leakage – does not require immediate recording of data





Evanescent Field Fiber Loop Ringdown (EF-FLRD)

- Change in water content will affect optical refractive index of fiber loop
- Reusable and thus can be used for monitoring – needs timely attention in collection of data





Strain Gauge Monitoring

- Can be used to measure expansion joint opening
- To assess prediction of joint opening (especially for Compression seal joint) and verify with actual joint opening
- Many ways to go about this – mature technology



Concluding Remarks

- Selection of compression seal or strip seal depends on various factors
- Block-out a very important parameter; Needs further research with elastomeric concrete, reinforced concrete, etc.
- Backwall recommended to be used – reduces source of leakage to only joint
- Monitoring needed to find out if poor construction is the cause of leakage or if inadequate design is the cause



University of Pittsburgh

Questions or Comments

Thank you

